UNCLASSIFIED

AD NUMBER AD859286 **NEW LIMITATION CHANGE** TO Approved for public release, distribution unlimited **FROM** Distribution authorized to U.S. Gov't. agencies and their contractors; Administrative/Operational Use; APR 1969. Other requests shall be referred to Naval Air Systems Command, Washington, DC. **AUTHORITY** USNASC ltr, 26 Oct 1971



Contract No. 00019-68-C-02 1

COLLOCATION FLUTTER ANALYSIS STUDY

This document is subject to special export controls and transmittal to foreign governments or foreign nationals may be made only with prior approval of the Naval Air Systems Command (AIR-350214).

ribofologo

VOLUME II.

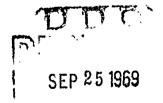
FLUENC - COMPUTER PROGRAM TO CALCULATE STRUCTURAL INFLUENCE COEFFICIENTS

APRIL 1969



MISSILE SYSTEMS DIVISION







COFA

COLLOCATION FLUTTER ANALYSIS

STUDY

VOLUME II

FLUENC - Computer Program to Calculate Structural Influence Coefficients

Prepared by the Dynamics and Environment Section Personnel, Hughes Aircraft Company Under Contract No. 0019-68-C-0247

April 1969

This document is subject to special export controls and transmittal to foreign governments or foreign nationals may be made only with prior approval of the Naval Air Systems Command

ABSTRACT

A displacement solution for the calculation of structural influence coefficients (SIC's) is presented. The formulation utilizes the lumped parameter approach that is consistent with collocation flutter solutions. The structure is synthesized as concentrated mass elements connected by massless elastic plates and/or beams. There are two methods of generating the mass matrix; they are: 1) lumped concentrated mass points, 2) consistent mass matrices. Along with the calculation of the SIC's, the natural vibration modes and frequencies are calculated. There are two options for punching out the flexibility matrix for use in subsequent COFA computer programs. Option 1, punches out the full flexibility matrix; Option 2, punches out the reduced flexibility matrix eliminating the rows and columns pertaining to structural attach points.

TABLE OF CONTENTS

													Pag€
	ABSTRAC	r	•		•	•	•	•	•	•	•	•	ii
	TABLE O	f Conten	TS.		•		•	•	•				i
1.0	INTRODUC	CTION .			•		•	•		•	•	•	1
2.0	NOMENCL	ATURE .	•		•	•	•	•	•	•		•	2
3.0	TECHNICA	AL DISCU	SSION		•	•		•	•		•	•	3
	3.1	Influen	ce Coei	ficiè	nts.	•	•				•		3
	3.2	Mass Ma	trix			•	•	•		•	•	•	12
	3.3	Modes a	nd Free	luenci	es .	•	•	•	•	•	•	•	15
4.0	PROGRAM	DESCRIP	TION				•		•	•	•		17
	4.1	Descrip	tion of	Prog	ram I	put	•	•			•		17
	4.2	Descrip	tion of	Prog	ram O	itput	•	•		•	•	•	22
	4.3	Sample	Problem	as ,	•		•		•				22
	4.4	Process	ing Rec	gu ire m	ents		•		•	•	•		23
	4.5	Program	Listi	ing an	d Flor	Cha	rt	•	•	•		•	23
REFERENCE	s		•		•	•		•	•	•			29
APPENDICE	S												
	APPENDI	X A	Three	Samp1	e Prob	lems	- Ir	put	and	Out	out		30
	APPENDI	ХВ	Progra	m FLU	ENC L	stin	B	•	•	•	•	•	81
	APPENDI	хс	Progra	m FLU	ENC F	low Cl	hart	•	•	•	•	•	126
	APPENDI	X D	Symbol	List			•		•	•	•	•	189
TABLES													
	1. Bea	nm Stiff	n ess Ma	trix									25
	2. Tr	iangular	Plate	Matri	x								26
	3. Bea	am Consi	stent M	iass M	atrix								27

1.0 INTRODUCTION

In order to determine the aeroelastic behavior of a wing or control surface, it is necessary to know the aerodynamics, elastic properties and mass distributions of the structure. The overall aeroelastic analysis is usually divided into four separate parts as shown in Figure 1.

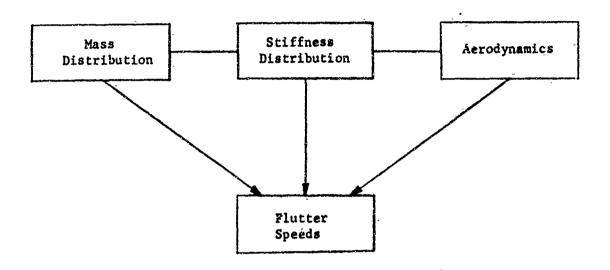


Figure 1. Analysis Procedure

This portion of the report describes the computation of the mass and stiffness distribution. The geometry of a wing or tail surface is too complex for the successful use of closed form analytical techniques. Therefore, a numerical type of analysis must be used. The end product of this analysis is the generation of overall influence coefficient and mass matrices referred to a set of node points arbitrarily picked on the surface of the structure. The finite element method (see Refs. 2 and 3) was used to form the required matrices for a planar structure. This technique is especially suited to solve complex structures and as used in the analysis is general enough to handle the following:

- 1. Combinations of beam and plate elements
- 2. Arbitrary boundary conditions
- 3. Lumped or distributed stiffnesses and masses

A discussion of the theory and computer program which calculates the influence coefficient and the mass matrix as well as the structural modes and frequencies is given in the following sections.

2.0 NOMENCLATURE

C = Unknown Boundary Constants

D = Plate Ridigity Constant

E = . Modulus of Elasticity

F = Force

K = Stiffness Coefficients

M = Bending/Torsional Moment

p = Pressure

T = Coordinate Transformation

t = Thickness

w = Linear Displacement in s direction

x, y, z = Coordinate Axes

s = Linear Displacement

 $\frac{c|\hat{C}|}{c(\sqrt{2})} = Curvature$

P = Density

T = Stress

7/ = Poisson's Ration

Partial Derivative

Square Matrix

Column Matrix

| | = Row Matrix

3.0 TECHNICAL DISCUSSION

3.1 Influence Coefficients

The stiffness method approach is first used to obtain an overall stiffness matrix of the structure. This matrix is reduced by partitioning and then inverted to obtain the influence coefficients at any desired set of control points. The number of control points are denoted by N. At each node, three degrees of freedom are specified: two rotations and the normal displacement. Therefore, a stiffness matrix of approximately 3N degrees of freedom is first formed by superimposing individual plate and plane grid beam element global coordinate matrices. The matrix will be somewhat smaller than 3N degrees of freedom since boundary restraint conditions will reduce the size of the matrix. To illustrate the matrix condensation method used in the computer program, we will assume that we have N control point normal displacements and M displacements which must be eliminated. The overall stiffness matrix is given as

$$[K] = \begin{bmatrix} K_{NN} & K_{NM} \\ K_{MN} & K_{MM} \end{bmatrix}$$
 (1)

The structural equilibrium matrix equation can be written as

$$\begin{bmatrix}
K_{NN} & K_{NM} \\
K_{MN} & K_{MM}
\end{bmatrix}
\begin{cases}
\delta_{N} \\
\delta_{M}
\end{cases} = \begin{cases}
F_{N} \\
F_{M}
\end{cases}$$
(2)

We now assume that forces at the points to be eliminated are small and can be neglected. Therefore,

$$\begin{bmatrix} K_{NN} & K_{NM} \\ K_{MN} & K_{MM} \end{bmatrix} \begin{Bmatrix} \delta_{N} \\ \delta_{M} \end{Bmatrix} = \begin{Bmatrix} F_{N} \\ O \end{Bmatrix}$$

or

$$[K_{NN}]\{\delta_N\} + [K_{NM}]\{\delta_M\} = \{F_N\}$$

and

$$[K_{MN}]\{\delta_N\} + [K_{MM}]\{\delta_M\} = \{0\}$$

Therefore

$$\left\{\delta_{\mathbf{M}}\right\} = -\left[K_{\mathbf{M}\mathbf{M}}\right]^{-1} \left[K_{\mathbf{M}\mathbf{N}}\right] \left\{\delta_{\mathbf{N}}\right\} \tag{3a}$$

and

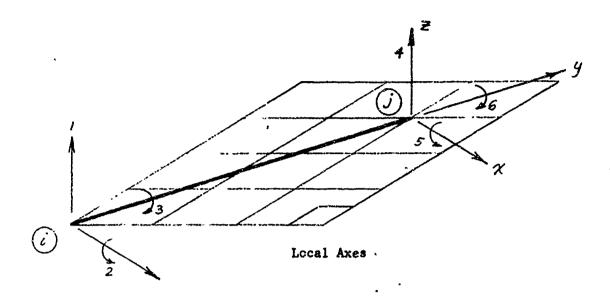
$$\left(\begin{bmatrix} K_{NN} \end{bmatrix} - \begin{bmatrix} K_{NM} \end{bmatrix} \begin{bmatrix} K_{NM} \end{bmatrix}^{-1} \begin{bmatrix} K_{MN} \end{bmatrix} \right) \left\{ \delta_{N} \right\} = \left\{ F_{N} \right\}$$
and since

$$\begin{bmatrix} K_{MN} \end{bmatrix}^T = \begin{bmatrix} K_{NM} \end{bmatrix}$$
we have
$$\begin{cases} S_N \\ = \left(\begin{bmatrix} K_{NN} \end{bmatrix} - \begin{bmatrix} K_{MN} \end{bmatrix}^T \begin{bmatrix} K_{MM} \end{bmatrix}^{-1} \begin{bmatrix} K_{MN} \end{bmatrix} \right) \begin{cases} F_N \\ \end{bmatrix}$$
If we now let
$$\begin{bmatrix} f_{NN} \end{bmatrix} = \left(\begin{bmatrix} K_{NN} \end{bmatrix} - \begin{bmatrix} K_{MN} \end{bmatrix}^T \begin{bmatrix} K_{MN} \end{bmatrix}^T \begin{bmatrix} K_{MN} \end{bmatrix}^{-1} \right)$$

then Equation (4) can be written as

$$\left\{ \delta_{N} \right\} = \left[f_{NN} \right] \left\{ F_{N} \right\} \tag{5}$$

The matrix $\begin{bmatrix} f_{NN} \end{bmatrix}$ is called the structural influence coefficient matrix. The application of loads at the control points yield displacements at the control points by carrying out the matrix multiplication indicated in Equation (5).



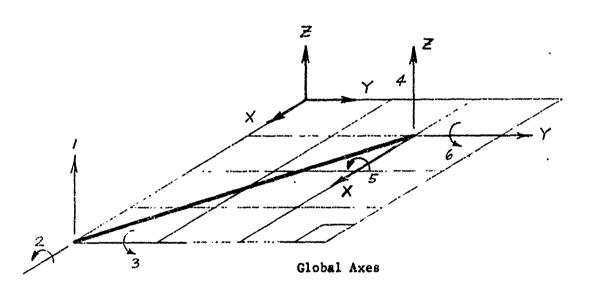


Figure 2. Plane Grid Beam Local and Global Coordinate System

The computer program FLUENC carries out the required operations to obtain the influence coefficient matrix $\begin{bmatrix} f_{NN} \end{bmatrix}$. A detailed description of the program can be found in Section 4.0. The program is written to form a 50 x 50 influence coefficient matrix. The influence coefficient matrix is punched out on cards in a format compatible with the Collocation Flutter Program.

The plane grid beam global coordinate stiffness matrix used in the program was obtained from Reference 1 and is given in Table 1. The local and global coordinate systems are shown in Figure 2. The figure also contains the sign convention for the six degrees of freedom for each element.

The triangular plate stiffness matrix given in Reference 2 was used in the computer program. The plate element can be materially or geometrically orthotropic as treated in Reference 3. Stiffened plates can be considered to be geometrically orthotropic. The sign convention and nodal degrees of freedom are shown in Figure 3.

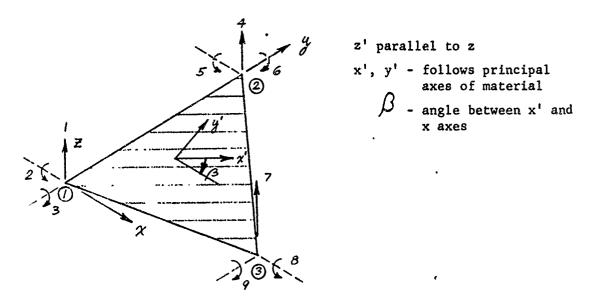


Figure 3. Orthotropic Triangular Element

Following the analysis given in Reference 2, the deflection shape of the plate element is assumed to be of the form

$$w = C_1 + C_2 x + C_3 y + C_4 x^2 + C_5 xy + C_6 y^2 + C_7 x^3 + C_8 (xy^2 + x^2y) + C_9 y^3$$

or

$$w = [N] \{C\}$$
 (6)

The unknown constants c_1 , c_2 , ---, c_9 can be written in terms of the nodal displacements δ_1 , δ_2 , ---, δ_9 by using the boundary conditions

at
$$\chi = 0$$
, $y = 0$

$$\begin{cases}
w = \delta, \\
\partial w / \partial y = \delta_2, \\
\partial w / \partial \chi = -\delta_3
\end{cases}$$
at $\chi = 0$, $y = y_2$

$$\begin{cases}
w = \delta_4 \\
\partial w / \partial y = \delta_5, \\
\partial w / \partial \chi = -\delta_6
\end{cases}$$
at $\chi = \chi_3$, $y = y_3$

$$\begin{cases}
w = \delta_7, \\
\partial w / \partial y = \delta_8, \\$$

Using Equation (6) in conjunction with the boundary conditions given by Equation (7) yields

$$\begin{cases}
\delta_{1} \\
\delta_{2} \\
\vdots \\
\delta_{3} \\
\delta_{4} \\
\delta_{5} \\
\delta_{6} \\
\delta_{7} \\
\delta_{8} \\
\delta_{9}
\end{cases} =$$

$$\begin{bmatrix}
\epsilon_{1} \\
\epsilon_{2} \\
\epsilon_{3} \\
\vdots \\
\epsilon_{n} \\
\epsilon_{n} \\
\vdots \\
\epsilon_{n} \\
\epsilon_{n$$

The constant vector $\{c\}$ can be obtained in terms of the nodal displacements by inverting the matrix [C]. Therefore,

$$\{c\} = \left[C\right]^{-1} \{s\} \tag{9}$$

The curvatures for a flat plate element are given by

$$\left\{ \epsilon \right\} = \left\{ \begin{array}{l} \epsilon_{\chi} \\ \epsilon_{y} \\ \epsilon_{\chi y} \end{array} \right\} = - \left\{ \begin{array}{l} \partial^{z} \omega / \partial \chi^{z} \\ \partial^{z} \omega / \partial \chi^{z} \\ 2 \partial^{z} \omega / \partial \chi \partial y \end{array} \right\}$$

Substituting Equation (6) into Equation (10) yields

$$\{\epsilon\} = [Q]\{c\} \tag{11}$$

where

Substituting Equation (9) into Equation (11) yields

$$\{\epsilon\} = [\beta] \{\delta\} = [B] \{\delta\}$$

If initial strains are neglected then the moment-curvature relationships can be written in the form

$$\{\sigma\} = \begin{cases} M_{\chi} \\ M_{y} \\ M_{\chi_{k}} \end{cases} = [D] \{\epsilon\}$$
(13)

where

$$\begin{bmatrix}
D_{x} & D_{x} & 0 \\
V_{x} & D_{y} & 0 \\
0 & 0 & D_{xy}
\end{bmatrix}$$
(14)

for a materially or geometrically orthotropic plate. For an isotropic plate Equation (14) reduces to

$$[D] - \frac{Et^3}{2(1-\nu^2)} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix}$$
 (15)

The $\left[D\right]$ matrix must undergo a transformation if the principal axes of the material do not coincide with the local coordinate axes. The components of strain in one coordinate axes system are related to the components of strain in another coordinate axes system by the matrix equation

$$\{\epsilon'\} = [T]^{\mathsf{T}}\{\epsilon\} \tag{16}$$

(The prime refers to the components of strain referred to the $x^{i}-y^{i}$ axes in Figure 3)

where

$$[T] = \begin{bmatrix} \cos^2 \beta & \sin^2 \beta & -2 \sin \beta \cos \beta \\ \sin^2 \beta & \cos^2 \beta & 2 \sin \beta \cos \beta \\ \sin \beta \cos \beta & \sin \beta \cos \beta & \cos^2 \beta & \cos^2 \beta \end{bmatrix}$$
(11)

Since the internal work is constant no matter which coordinate system is used

$$\left\{\sigma'\right\}^{\mathsf{T}}\left\{\epsilon'\right\} = \left\{\sigma\right\}^{\mathsf{T}}\left\{\epsilon\right\} \tag{18}$$

or by Equation (13)

$$\{\epsilon'\}^{\tau}[D']\{\epsilon'\} = \{\epsilon\}^{\tau}[D]\{\epsilon\}$$

and by using Equation (16)

$$\{\epsilon\}^{\mathsf{T}}[\mathsf{T}][\mathsf{D}'][\mathsf{T}]^{\mathsf{T}}\{\epsilon\} = \{\epsilon\}^{\mathsf{T}}[\mathsf{D}]\{\epsilon\}$$

Therefore

$$[D] = [T][D'][T]^{T}.$$
(19)

The stiffness matrix for a typical element ① ② ③ is given by

$$[K] = \iint_{A} [B][D][B] dx dy$$
(20)

or by Equations (12) and (19)

$$[K] = [C^{-1}]^{T} \left(\iint_{A} [Q]^{T} [T] [D^{\prime}] [T]^{T} [Q] dx dy \right) [C]^{-1}$$
(21)

Now let

$$[\bar{D}] = \iint_{\Omega} [Q]^{T} [T] [D'] [T]^{T} [Q] dx dy$$

and carrying out the indicated matrix multiplications yields

$$\left[\widetilde{D}\right] = \iint_{A} \left(see \ Table \ /\ \right) dy dy \tag{22}$$

In order to simplify the integration required for evaluating the matrix in Equation (22), it is suggested in Reference 2 that the independent variables be changed as shown in Figure 4.

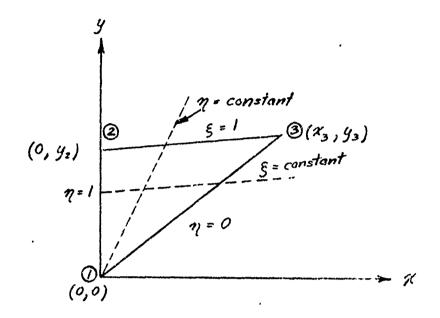


Figure 4
Coordinate Transformation

The relationships

$$x = \xi (1 - \eta) x_3$$

$$y = \xi [(1 - \eta) y_3 + \eta y_2]$$
(23)

are used for the change of variables. The terms in Equation (22) can now be evaluated by using the relationship

$$I(x^m, y^n) = \iint x^m y^n dx dy$$

or

$$I(x^m, y^n) = \iint x^m y^n \left| J(x, y) \right| dx d\eta \tag{24}$$

where

$$J(x,y) = \begin{vmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial y}{\partial \eta} \\ \frac{\partial y}{\partial \xi} & \frac{\partial y}{\partial \eta} \end{vmatrix}$$
(25)

Substituting Equation (23) into Equation (25) yields

$$\mathcal{J}(x,y) = \xi \chi_3 y_2 \tag{26}$$

(27)

Substituting Equations (23) and (26) into Equation (24) yields

$$I(x^{m}, y^{n}) = \int \int \xi^{m+n+1} (1-\eta)^{m} [(1-\eta)^{y_{3}} + \eta y_{2}]^{n} \chi_{3}^{m+1} y_{2} d\xi d\eta$$

which can easily be evaluated for any m and n.

3.2 Mass Matrix

D'Alenbert's principle can be used for the formulation of the mass matrix. If masses are attached to the nodes of the structure, then the nodal dynamic forces are

$$\left\{P\right\} = -\left[M\right] \frac{d^2\left\{\delta\right\}}{d\xi^2} \tag{28}$$

where

$$[M] = \begin{bmatrix} M_1 & 0 \\ M_2 & \\ 0 & M_n \end{bmatrix}$$

is a diagonal matrix. The mass of beam and plate elements are usually distributed throughout the structure. Therefore, the distributed pressure loading can be written in the form

$$p = -p \frac{d^2 w}{dt^2} \tag{30}$$

Substituting Equations (6) and (9) into Equation (30) yields

$$p = -p[N][c]^{-1}\{\ddot{s}\}$$

or

$$P = -\rho [R] \{ \ddot{\delta} \} \tag{31}$$

where

$$[R] = [N][C]^{-1}$$

Since the equivalent element nodal forces can be computed from the equation

$$\left\{P\right\}^{e} = -\int_{V} \left[R\right]^{T} p \ dV \tag{32}$$

then

$$\left\{ P \right\}^{e} = \left\{ \int_{V} \left[R \right]^{T} \left[R \right] \rho \, dV \right\} \left\{ \ddot{S} \right\} \tag{33}$$

Therefore the elemental consistent mass matrix is given by

$$[m]^{e} = \int [R]^{T} [R] \rho dV \tag{34}$$

The consistent mass matrices given in Reference 2 (see Tables 3 and 4) are used in the computer program.

Once the elemental consistent mass and/or lumped mass matrices are computed, then the overall matrix is obtained by following the same technique as used in assembling the overall stiffness matrix.

The overall mass matrix is reduced by using Equation (3a). We again assume that we have N control point normal displacements and M displacements which must be eliminated. The overall mass matrix can be written in the form

$$\begin{bmatrix} M \end{bmatrix} = \begin{bmatrix} M_{NN} & M_{NM} \\ M_{MN} & M_{MM} \end{bmatrix}$$
(35)

and the displacements

$$\left\{ \mathcal{S} \right\} = \left\{ \begin{array}{c} \mathcal{S}_{N} \\ \mathcal{S}_{M} \end{array} \right\} \tag{36}$$

From Equation (3a) we have

$$\{\delta_{\mathbf{M}}\} = -[K_{\mathbf{M}\mathbf{M}}][K_{\mathbf{M}\mathbf{N}}]\{\delta_{\mathbf{N}}\}$$

Since the virtual work of the reduced mass system must equal the virtual work of the true mass system

$$-\left\{\Delta \delta_{N}\right\}^{T}\left[M_{r}\right]\left\{\ddot{\delta}_{N}\right\} = -\left\{\Delta \delta\right\}^{T}\left[M\right]\left\{\ddot{\delta}\right\} \tag{37}$$

where

$$\left\{ \Delta \delta_{N} \right\}$$
 = virtual displacements of control points $\left\{ \Delta \delta \right\}$ = virtual displacements of complete system $\left[M_{r} \right]$ = overall reduced mass matrix

Equation (37) can be rewritten in the form

$$\{\Delta \delta_{N}\}^{T}[M_{r}]\{\delta_{N}\} = [\Delta \delta_{N}^{T} \Delta \delta_{M}^{T}][M]\{\delta_{N}\}$$
(38)

Substituting Equation (3a) into Equation (38) yields

$$\left\{\Delta \delta_{N}\right\}^{T}\left[M_{r}\right]\left\{\ddot{\delta}_{N}\right\} = \left\{\Delta \delta_{N}\right\}^{T}\left[I - \left[K_{NM}\right]\left[K_{MM}\right]^{-1}\right]\left[M\right]\left[I - \left[K_{MM}\right]^{-1}\left[K_{MN}\right]\right]\left\{\ddot{\delta}_{N}\right\}\right]$$

which yields the result

$$|M_{r}| = \left[I - \left[K_{NM}\right]\left[K_{MM}\right]^{-1}\right]\left[M\right]\left[K_{MM}\right]^{\prime}\left[K_{MN}\right]^{\prime}$$
(39)

The reduced mass matrix given by Equation (39) is calculated in the computer program.

3.3 Modes and Frequencies

Since the design engineer may find it useful to know the mode shapes and natural frequencies of the structure, this information can be obtained by using the NMØDE option in the computer program. If no external forces are present then the reduced mass and influence coefficient matrices are related to one another by the relationship

$$\left[f_{NN}\right]^{-1}\left\{\delta_{N}\right\} = -\left[M_{r}\right]\left\{\ddot{\delta}_{N}\right\} \tag{40}$$

For determining natural frequencies, the deflections $\left\{\delta_{\mathcal{H}}\right\}$ can be written as

$$\{\delta_{N}\} = \{\delta_{o}\} \sin \omega t \tag{41}$$

Substituting Equation (41) into Equation (40) yields

$$[f_{NN}]^{-1} \{ \delta_o \} = \omega^2 [M_r] \{ \delta_o \}$$
(42)

The solution of Equation (42) yields the natural frequencies, α , and the mode shapes $\left\{\delta_{o}\right\}$. Since $\left[\mathbf{f}_{NN}\right]^{-1}$ and $\left[\mathbf{M}_{r}\right]$ are both symmetrical matrices, the mass matrix $\left[\mathbf{M}_{r}\right]$ can be triangularized

$$[M_r] = [L][L]^T$$
(43)

where

$$[L] = \begin{bmatrix} l_{11} & 0 & 0 - - - - 0 \\ l_{21} & l_{22} & 0 - - - - 0 \\ \vdots & \vdots & \vdots \\ l_{n1} - - - - - - & \vdots \\ l_{nn} \end{bmatrix}$$

Substituting Equation (43) into Equation (42) yields

$$[f_{NN}]'\{S_o\} = \omega^2[L][L]^T\{S_o\}$$

$$[L]^{-1}[f_{NN}]^{-1}\{\delta_{\bullet}\} = \omega^{2}[L]^{T}\{\delta_{\bullet}\} \tag{44}$$

Since

$$\left[L^{T}\right]^{-1}\left[L^{T}\right] =\left[I\right]$$

Equation (44) may be written

$$[L]^{-1}[f_{NN}]^{-1}[L]^{-1}[L]^{-1}[\delta_{0}] = \omega^{2}[L]^{-1}[\delta_{0}] \qquad (44e)$$

or

$$[A]\{\bar{S}_o\} = \omega^2\{S_o\}$$
(45)

where
$$\begin{bmatrix} A \end{bmatrix} = \begin{bmatrix} L \end{bmatrix}^{-1} \begin{bmatrix} f_{NN} \end{bmatrix}^{-1} \begin{bmatrix} L^{T} \end{bmatrix}^{-1}$$

$$\{ \overline{\delta}_{c} \} = \begin{bmatrix} L \end{bmatrix}^{T} \{ \delta_{c} \}$$

An eigenvalue subroutine using the Givens method was used in the computer program package to solve Equation (45). The Givens method is fully described in Reference 4.

Note that the dynamical matrix [A] in the form described above is real and symmetric which is required by the Givens method. Conveniently, [L] and $\begin{bmatrix} L^T \end{bmatrix}$ are in triangular form which is used in the computer program package to save core storage space.

4.0 PROGRAM DESCRIPTION

Computer program FLUENC written in FORTRAN IV carries out the operations set forth in Section 3.0 for generating the structural influence coefficients and mass matrices required by the Collocation Flutter Program. Briefly, the structure is assumed to be representable by a planar network of beams and triangular plate elements connected at discrete joints. At each joint, if there are no restraints, the program assumes three degrees of freedom; that is, one displacement normal to the plane of the structure and two rotations. The program first synthesizes the stiffness and mass matrices for the entire structure including all degrees of freedom from the data input for the beam and triangular plate elements and from the restraint information input for the joints. It then reduces the stiffness and mass matrices by eliminating all the rotational degrees of freedom and leaving only the normal displacements. As a final step, the program inverts the reduced stiffness matrix to obtain the influence coefficients.

Other features of the program include the option to compute lumped masses or to compute the consistent mass matrices for the beam and triangular plate elements or both. Also, the triangular plate elements may have either isotropic or orthotropic properties. There is an additional option to expand the reduced frequency matrix to include the degrees of freedom representing the restraint joint (one joint on a movable surface; two joints on a fixed component). This is accomplished by adding one or two zero rows and columns to the reduced flexibility matrix corresponding to the mass numbers of the attach points involved.

In the sections that follow detailed instructions are given for the preparation of input data and a description is given of the output illustrated with several sample problems. Also included are listings and flow charts of the program and a discussion of the processing requirements.

4.1 Description of Program Input

The following instructions describe the input data, their physical units, and the FORTRAN format they must be punched with. The input quantities' names, all in capitals, are their FORTRAN names and, for reference, their equivalent names in Section 3.0 are listed in Appendix D.

4.1.1 Title Card, format (12A6)

Two cards; any alphanumeric statement in columns 1 to 72.

4.1.2 Problem Size and Control Information, format (715)

Column	1 - 5	6 - 10	11 - 15	16 - 20	21 - 25	26 '- 30	31 - 3
Name	njts	NR	NBE	npe	nmøde	MKEY	NLUMP
							163

NJTS = number of joints in structure (50 maximum)

NR = number of joints with one or more restraints

NBE = number of beam elements in structure

NPE = number of triangular plate elements in structure

NMØDE = number of eigenvalues and eigenvectors desired (9 maximum)

MKEY = 1. do not compute consistent mass terms for beam and/or triangular plate elements

= 2. compute consistent mass terms for beam and/or triangular plate elements

NLUMP = number of lumped masses input. Only lumped masses corresponding to the normal displacement at each joint may be input.

4.1.3 Material Properties

(a) Number of Materials, format (15)

Column 1 - 5
Name NMAT

NMAT = number of materials for which properties are input (10 max.)

(b) Properties, format (4E10.3)

Input NMAT number of cards, one for each material.

-				
Column	1 - 10	11 - 20	21 - 30	31 - 40
Name	YM(1)	PR(1)	GE(1)	DENS (1)

YM(i) = Young's modulus of elasticity divided by 106; psi

PR(i) = Poisson's ratio

GE(i) = modulus of rigidity; psi. If input as 0, it will be computed from the following formula:

$$GE(i) = \frac{YM(i)}{2 \left[1 + PR(i)\right]}$$

DENS(i) = material density; 1b/in³. Not required if MKRY = 1

4.1.4 Joint Coordinate Cards, format (10X, 2E10.3)

Input NJTS number of cards, one for each joint. Also, the structure is assumed to lie in the x-y plane.

Column

Name

-	1 - 10	11 - 20	21 - 30
-	m	X (m)	Y (m)

m = joint number (must be input consecutively starting with 1).

May be placed anywhere between columns 1 and 10

X(m) = x coordinate of joint m; inches

Y(m) = y coordinate of joint m; inches

4.1.5 Joint Restraint Information, format (415)

Input NR number of cards, one for each joint with one or more restraints.

Column

Name

1 - 5	6 - 10	11 - 15	16 - 20
JT	Ml	M2	м3

JT = number of joint having one or more restraints

M1 = 0 free in the z direction

= 1 fixed in the z direction

M2 = 0 free to rotate about the x axis

= 1 fixed about the x axis

M3 = 0 free to rotate about the y axis

= 1 fixed about the y axis

4.1.6 <u>Lumped Masses</u>, format (15, 5X, E10.3)

Input NLUMP number of cards, one for each lumped mass.

Co1umn

Name

1 - 5	6 - 10	11 - 20
JMASS	blank	RSMASS

JMASS = number of joint for which lumped mass is input

RSMASS = lumped mass, 1b.

If more than one lumped mass is input for a particular joint, the program will sum the masses.

4.1.7 Beam Element Properties, format (3E10.3, 3I5)

Input NBE number of cards, one for each beam element.

Column	1 - 10	11 - 20	21 - 30	31 - 45	36 - 40	41 - 45
Name	AR	XI	YJ	MAT	JTNR	JTFR

AR = area of beam cross section, in^2

XI = moment of inertia of area, in4

YJ = effective torsional moment of inertia, in 4

MAT = material code corresponding to one of the materials input under paragraph 4.1.3.

JTNR, JTFR = joint numbers at the ends of the beam element

4.1.8 Triangular Plate Element Properties, format (E10.3, 515)

Input NPE number of cards, one for each triangular plate element.

Column	1 - 10	11 - 15	16 - 20	21 - 25	26 - 30	31 - 35
Name	РТН	MAT	JTl	JT2	JT3	NDX

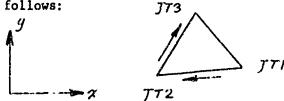
PTH = plate thickness, in.

MAT = material code corresponding to one of the materials input under paragraph 4.1.3

JT1, JT2, JT3 = joint numbers at the three corners of the triangular plate

Restrictions:

a) The order of the joint numbers must be given in a clockwise manner as follows: 773



- b) The angle formed by the edges of the triangular plate at JT1 must not be 90°.
- NDX = 0 the plate has isotropic properties and the flexural rigidity terms are computed from

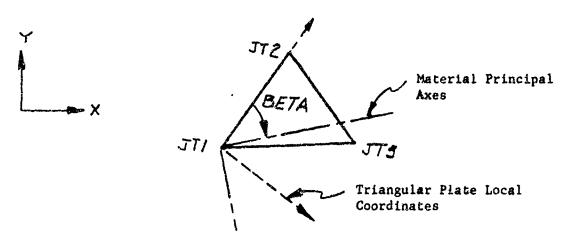
$$DX = DY = \frac{YM(MAT) \times PTH^3}{12\left\{1 - \left[PR(MAT)\right]^2\right\}}$$

the place has orthotropic properties and the flexural rigidity terms are input by the next card [format (4E10.3)]

Column	1 - 10	11 - 20	21 - 30	31 - 40	41 - 50
Name	DX	DY	D1	DXY	BETA

DX, DY, D1, DXY = flexural rigidity terms, in.1b.

BETA = angle between material principal axes and the triangular plate local coordinates as shown below



4.1.9 Option to Expand Reduced Flexibility Matrix

Note: The following card (NCOD) is always required at the end of all input data for any one particular case, whether or NOT the option is to be executed.

	FORMAT ([16]
Column	1-6	
Name	NCOD	
ltem	(1)	

NCOD = 0 Option not executed

= 1 Option executed

If NCOD = 1, the following card is required

	FORMAT (318)		
Column	1-8	9~16	17-24
Name	NR	NNE	NWO
Item	(1)	(2)	(3)

NR = Number of boundary points used (1 or 2)

NNE = Mass number of first attach point

NWO = Mass number of second attach point, if NR = 2

NWO = 0 or left blank if NR = 1

To input more than one problem, the user need only repeat the cards in paragraph 4.1.1 through 4.1.8 for each additional problem.

4.2 Description of Program Output

The program prints out all the input data for every problem followed by the solution consisting of the reduced upper right triangular stiffness (lb/in), flexibility (in/lb) and weight (lb) matrices as well as the modes and frequencies when these are requested on the card in paragraph 4.1.2. The stiffness, flexibility, and mass matrices that are printed/punched out only contain terms that are associated with the normal displacement "z". This is done so that when the flexibility matrix is used in subsequent collocation flutter analyses only the essential degrees of freedom are included in the flutter analyses. Also, the matrices are reduced to eliminate control points associated with fixed points (boundaries). If it is desirable to include the boundary points, it is only necessary to intersperse rows and columns of zero's at the proper place in the matrices. Immediately following the joint restraint information in the output, the program prints out the coordinate numbers assigned by the program to the normal displacements at each unrestrained joint. The elements in all the reduced output matrices are ordered according to these coordinate numbers.

In addition, the program punches out the entire flexibility and weight matrices row by row with the format (1P6E12.5) which is compatible with the input requirements of the Collocation Flutter Program. Each punched matrix is identified by a little card as the first card.

4.3 Sample Problems

To illustrate the use of program FLUENCE, three sample problems are included in Appendix A. Each sample problem starts with a problem statement and is followed by a listing of the input data and the output of the program. The first sample problem is a simply supported uniform beam composed of five beam segments. The second is a uniform cantilever plate divided into 72 triangular plate elements, and the third is a lumped mass and beam network simulating a missile control surface.

4.4 Processing Requirements

Program FLUENCE has been run on the GE-635 computer and it required about 31,000 cells of core storage. It is expected that the program storage requirement will be about the same on other digital computers. In addition to using the input and output files, 05 and 06, which are standard for the GE-635 computer, the program requires six other peripheral files, five of which are designated in the program by the numeric codes 07, 08, 19, 10 and 11, and the sixth is the card punch file.

There is no general formula for determining the run time required for a problem, but if a GE-635 computer is used, an estimate may be made from the times required for the three sample problems in Appendix A, which are as follows:

Sample Problem No.	No. of Joints	No. of Beam Elements	No. of Plate Elements	Consistent Masses Computed	Lumped Masses Input	No. of Modes & Freqs Computed	Run Time Hr.
1	6	5	0	Yes	No	4	0.0015
2	50	0	72	Yes	No	9	0.0691
3	29	45	0	No	Yes	9	0.0161

4.5 Program Listing and Flow Chart

In the event future changes are needed in the program, a listing of the program is included in Appendix B. The program consists of a MAIN deck, 24 subroutines and one function subprogram. MAIN has the function of reading in data, numbering the coordinates (subroutine COORDN), generating the codes for assembling the stiffness and weight matrices and calling the subroutines which develop the stiffness and mass terms for the beam and triangular plate elements. When the entire stiffness and weight matrices have been established for the whole structure, the MAIN program calls a subroutine which reduces these matrices as discussed before and determines the modes and frequencies as well.

The 24 subroutines and one function subprogram can be divided conveniently into five groups according to their function. The first group consists of those routines that develop the beam stiffness terms; these are TRANS and BEAMK. The second group consists of the routines which determine the beam mass terms; these are TRANS and BEAMM. The third group develops the triangular plate stiffness terms and these are PLATEK, CMAT, MINV, DINMAT, MATMPY, DMAT, DBLINT and PLYMP. The fourth group determines the triangular plate mass terms and these consist of PLATEM, CMAT, MINV; DINMTM, MATMPY, DBLINT and PLYMP. The fifth group of subroutines reduces the stiffness and

and mass matrices, finds the eigenvalues and eigenvectors and outputs the solution. This group is comprised of EIGEN, VIVID, ZRØMAK, ZRØMAM, SYMINV, EIGMAT, BIGMAT, LØØP1, LØØP2, LØØP3 and LØØP4.

Since the program listing is annotated extensively with comment statements, no further explanatory remarks are given here for the program. However, to facilitate the understanding of the interrelationships among the many subroutines, a flow chart of the entire FLUENC program is included in Appendix C.

y					
		·			4EI 17 67 m
				4EIm - 6IL	-4EI for + GI for 4EI & + GI m2
	Symmetric	,	12EI L*	- <u>GEI</u> m	7 139
		65 fm 4EI 1 1 67 m2	SET A	67 L - 2EI CM - GT CM	-676 2EIL - 61m2
	4EIm. 4 6IL	-4EI In + GI In	m <u>17</u>	2EIm 2-67 62	-2EI Lm-67 Cm
12 E I	w <u>7799</u>	Y = 7 -	- <u>12£1</u> - 13 £ 1	m <u>179</u>	7 139 -

Parent I

X	
1 -	
X	
l	
Y	

$$\frac{7}{2\lambda-2\lambda}=w$$

 $X_1,\ Y_1,\ X_j,\ Z_j$ are the global end coordinates of the beam in Figure 2

Table 1. Plane Grid Beam Stiffness Matrix in Global Coordinates

5

-								
0	0	0	12 De y	120my	120.7	36 Dan	12[044+0m79 +20m(79+44)]	340 g
0	0	0	4[0,y+0,x+20,s(x+y)]	+[0,3.0,x.20,(x.y)]	4[0, y + 0, x + 20, (r y)]	12x[0,9.0,x+20,(x+y)]	$(2(D_{a}gy + D_{a}x^{2}) + 4y[D_{a}y + D_{a}x + 2D_{a}(x + y)] + 24D_{a}x + 2D_{a}x + 2D_{a}(x + y)] + 6(x + y)[D_{a}y + D_{a}x + 2D_{a}(x + y)] + 6(x + y)[D_{a}y + D_{a}x + 2D_{a}x + $	[(6,x) 02.x.20 (x.4)]
0	0	0	120, *	12Dyx	12 Du x	340,76	12 (0,34+0,12) + 240,1 (x+4)	360,77
0	0	0	40°	40n	40x	120ax	4(Dmy + Dmx) - 6Dm (x+y)	/ZDny
0	0	0	4D,	40n	4025	120° x	4(0,y+0,x) •60,x(x+y) •60,x(x+y)	1200.9
0	0	0	4D,,	403,	40.	120,7	4 (0.4.0.x) .80x (x.y)	120my
0	. 0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0

ចំ ត់តំ
0,0,0,
0 0 0
It
[0]
where

whis 2. Integrand Appearing in Equation 22. Intengular Finte Ristent

	· · · · · · · · · · · · · · · · · · ·				-
					34
				$\frac{L^2}{105} + \frac{2I}{15A}$, 0
	Symmetric		13 + 61 35 + 54L	$-\frac{IIL}{210} - \frac{I}{10AL}$	0
		J 34	0	0	7
	12 + 21 105 + 15A	0	$\frac{13L}{420} - \frac{L}{10AL}$	$\frac{408}{I} - \frac{041}{27}$	0
13 + <u>61</u>	$\frac{11L}{210} + \frac{I}{10AL}$	0	$\frac{9}{70} - \frac{6I}{5AL}$	$-\frac{13L}{420} + \frac{I}{10AL}$	0
,		r	[m] = pAL	27	

Consistent Mass Matrix for Beam in Local Coordinates Table 3.

Table 4. Consistent Mass Matrix for Triangular Plate Blement in Local Coordinates

 $[m] = \rho t [c^{-1}]^T$

References

- 1. Tezcan, S. S., "Computer Analysis of Plane and Space Structures", Journal of the Structural Division, ASCE, April 1966
- 2. Przemieniecki, J. S., "Theory of Matrix Structural Analysis", McGraw-Hill Book Co., New York, 1968
- 3. Zienkiewicz, O. C., "The Finite Element Method in Structural and Continuum Mechanics", McGraw-Hill Publishing Company Limited, London, 1967
- 4. Bishop, R. E.D, Gladwell, G. M. L., and Michaelson, S., "The Matrix Analysis of Vibration", Cambridge University Press, London, 1965

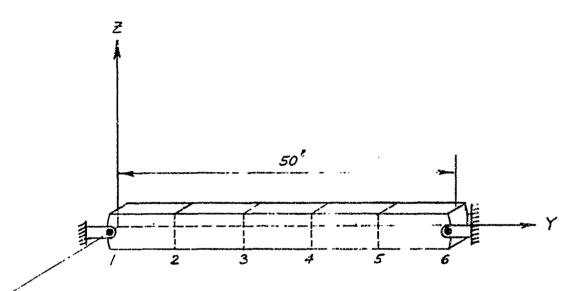
APPENDIX A

Three Sample Problems - Input and Output

(Class

Sample Problem No. 1 Simply Supported Beam

and the same



 $E = 10^6$ psi

v = 0.33

 $P = 0.012 \, \text{lb/in}^3$

 $A = 100 in^2$

 $I = 2 in^4$

 $J = 4 in^4$

Calculate first five vibration modes and frequencies using the consistent mass matrix option.

Listing of Input Data Sards

				A	
SIMPLY AUGUST	SUPPORTEN	BEAN WITH	A JOINTS	**)
6 2	· 5	9 4	Z		MON. REPROPRIE
1					
1.	B. 3.4	0.	ë.ni:		*(D).
1	9.	作。			The state of the s
.79	ti 🕳	10.			
4	0.	7 U .			
1	0.	18.			
5,	a •	44.			1
٨	n.	"1 (] .			
1	, 0	1			
₹.	Y Ø	1			
inn.	7.	A.	1	4	?
ing,	7.	ė,	1	C	3
1 110.	7.	4.	1	\$	4
1 49.	7.	٠.	1	4	ን
169.	7.	4.	1	7	6

Program Output

MIUNP .	
MKEY s 2	
NHODE # 4	
NPE . O	
NBE = 5	
NR = 2	
NJTS . 6	

C

*******	DENSITY 0.12000E-01
PROPERT FR S eseccescos secces	HODULUS OF MIGIDITY 0.37594E 06
0 P E R T M E S •	POISSON RATIO 0.33000
MATERIAL PR	YOUMBAS MODULUS 0.10000E 07
< X	Š. 4

NATES TCOORD.	4 4 4 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
COORDOX XCCCRO.	
JOINT NO.	in n e gre

; ;

į

. . . .

R E S T R A 1 I Z DISPLACEMENT

COORINATE NUMBERS FOR EACH 2 DISPLACEMENT AT EACH UNRESTRAINED JOINT JOINT NO. COORD. NO.

********	JOINT &	~	P)	→	16	•
*********	S THEORY & COERT &	**	W	n	₩	ın
****	HAH		-1	₩	~ 1	-1
TES **	7	4,0000	4.0000	4.0000	4.0000	4,0000
			2.0000		2,0000	2.6800
10年 10 11 11 11 11 11 11 11	٠	100,6000	100.000	100,0000	100.000	100.000
0 E M	ELENENT NO.		~	n	•	5 0

TRIANGULAR EPUCED

8.82879E 04 +0,20470E 04 -0.19005E 05 ROW 1 0.197518 .05

(

.

ROM 1 0.53333E-03 0.75000E-03 0.66667E-03 8.3833E-03 0,93900E 80 -8,56295E 86 6,21460E 80 0,11333E-02 0,66567E-03 0.11537E 01 -0.56295E 00 0.79000E-03 9,9390GE 80 UPPER ROW 2 0.12000E-02 . ROW 3 0#12000E-02 46 80W 1 172E 02 - 408 2 PO-BRRRRE-04 ROW 4 0,19751E 05 REDUCED REBUCED

ROW 3 0728019E 05 -0,19005E 05

HERE ARE THE EIGENVALUES AND EIGENVECTORS

i i

:

CORRESPONDING TO 1,0030593E 04 6.1803418E-01 EIGENVECTOR NUMBER 6-1803364E-01

EIGENVECTOR NUMBER 2 CORRESPONDING TO 1.6120593E 05 1.0000000E 00 6.1603410E-01 -6.1803363E-01 -9,999968E-01

EIGENVECTOR NUMBER 3 CORRESPONDING TO 4.478930E 05 1.6080080E 00 -6.1803399E-01 -6.1803461E-01 1.009080E 00

EIGENVECTOR NUMBER 4 CORRESPONDING 70 2,9656634E 06 -671863307E-01 1.0008800E 08 -9,999993E-01 6,1863399E-01

HERE ARE THE NATURAL PRECUENCIES

ļ

1

NATURAL PREDUENCY NATURAL PREDUENCY NATURAL PREDUENCY FREGUENCY

444

;

Ċ.

O

36 C

O

0

•;

~' •

()

C

: : Ĺ

Ċ

-0

:

SAMPLE PROBLEM NO. 1a

Simply Supported Beam

Identical to Sample Problem 1 with the addition of lumped mass input at joint 3 and 4.

Program Output

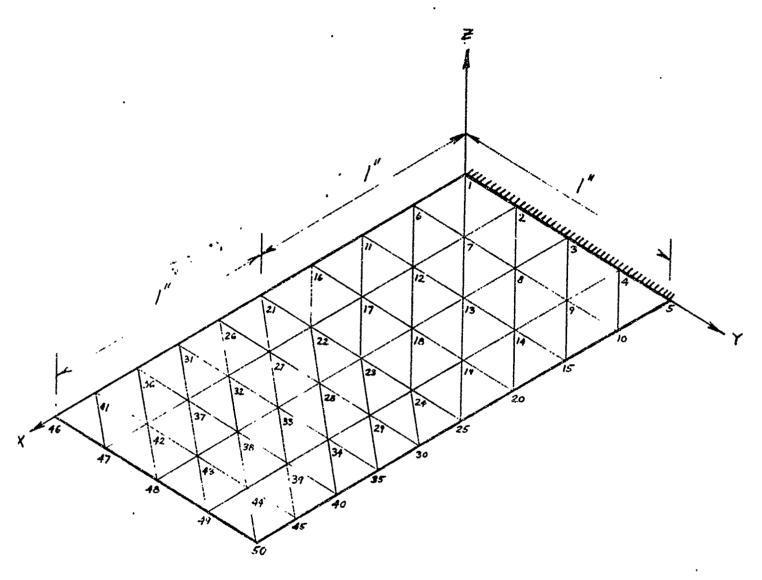
SIMPLY SUPPORTED BEAM WITH 6 JOINTS - USING BOTH CONSISTENT MASS MATRIX OPTION AND LUMPED MASS INPUT AT JOINTS 3 AND 4.		
NJTS = 6 NPE = 0 NHODE = 4 MKEY = 2 NLUMP = ?		
MATERIAL PROPERTIES ************************************		
NO. X COORD.		
3 6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		
0 N		
38		
COORINATE NUMBERS FOR EACH Z DISPLACEMENT AT EACH UMRESTRAINFD JOINT JOINT NO. COORD. NO. 1 3 3 7 7		
I U M P F D A E I G H I S JOINT NO. WEIGHT		
30.		
REALM FLEMENT PROPERTIFS ************************************		•
104.0000 2.0004 4.0004 1 1 2 2 3 4 4 0000 1 1 3 4 4 0000 1 1 1 3 4 4 0000 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
กับ. วาทีเกิน ว. กันกับ	,	
o F h ii G E h I i P D F b I T N R II N R R R T I F F N E S S N L T R T X	1	
		^

		_	Õ	^	^		^		<u> </u>	~	^		^	. ~	^	_	•	^	^	
				1		, , ,	- 1	1 1					1							
		i				decoupling											-			
		, ,	‡ ‡																	
		; ;																		
			and the second																	
			ŧ		T R 1 X	,														
			; ;		A H						HATRIX									
		- office about the	ļ		81117						N A T									
			; ,		E x 1 6						1 0 H I		***************************************				-			And the second second second
	u. 62679E a4 - 6.2h678E n4				1 4	0.38333E-03					S S	0.21468E au					:			
	04 - 0.21	4.	,		6 U L A	1	n3	***************************************			G_U_A_		0.0							,
	.87679E	1.82679E 0.4			N A N	3.66567E-03	11.66667E-03	•			T.F. I.A.M.G.U.L.A.R.	-11.56295E 110	-1.56295E 00				1	-		1
]	 18.	ž.	F 15		2	i	.]	JE-43				1	1 1	9E 10			1			•
	-0.19065E	-1.21072E	-4.30005		U P P E	6.7500UE-03	0.113338-02	0.75000E-03			UPPER	0.93900E 10	0-1153ZE 11	9.939006 10				!	:	1
		6.28019E n5	n 5	751E 05	n	1 0.53333E-03	1 2 0.12000E-02	ROW 3 0.12000E-02	1 4 0.53333E-03		DUCED	n.11172E n2	0.36609E.02.	50 36 US	0.11172E 02					
	ROW 1 1 0751E 05	ROW 2	804 3 0.28019E	80W 4 0.19751E	9 7 9 11 6 8 9	POW 1 0.533	ROW 2	ROW 3	ROW 4		e: w	.ROW.	ROW 2	ROW 3	ROW 4 0.11	•			í 1 1	
	<u> </u>		_					- {	_	3	9	-						-		_

						\mathcal{O}					
HERE ARE THE ELGENVALUES AND ELGENVECTORS	FIGENVECTOR WOMBER 1 COMPESPONDING TO	FIGENVECTOR WINRER 2 CORRESPONDING TO 1.0007129E n5 1.000000E 02 7.25,39527F-01 -5.665892E-01 -8.8324765E-01	FIGENVECTOR NOMBER 3 CORRESPONDING TO 6./652134E n5 8.9313311E-01 -1.9308592F-01 -1.8372997E-01 1.0004000E 00	FIGENVECTOR WOMBER 4 CORRESPONDING TO 1.32953650E n6 1.0000010F n0 -5.38947/2F-01 3.9625669F-01 -0.19819N7E-01	HEOF ADE THE MATURAL FREQUENCIES	THE NATURAL FREQUENCY NUMBER 1 15 10.054 CPS THE NATURAL FREQUENCY NUMBER 2 15 50.347 CPS THE NATURAL FREQUENCY NUMBER 3 15 130.906 CPS THE NATURAL FREQUENCY NUMBER 4 15 183.226 CPS				The state of the s	

Sample Problem No. 2

Cantilever Plate



 $E = 3 \times 10^7 \text{ psi}$

V = 0.3

 $\rho = 0.283 \text{ lb/in}^3$

t = 0.1 in.

Listing of Input Data Cards

CANTILE AUGUST		F WITH 50	JOINTS	
50	5 (I	77 9	2	U
1				
30.	0.3	0 •		4.285
1	0.	H .		
2	n .	.25		
.3	0.	٠,6		
4	0.	.75		
5	0.	٤.		
6	• 25	(I •		
7	. 75	.25		
8	• 25	• 2		
g 	• 25 05	.75		•
{ 1	• 25	1.		4YO'2
11	• 5	. 0		1. P.r.
15	• າ • າ	. 25 . 3		The state of the s
į 4	• 5	./5		10D17
15	• 7	1.		NOT REPRECOUCIELE
16	. 15	. 1)		S.L.B.
17	. 15	.25		•
: 8	. 75	. 5		
19	• /5	.15		
20	. 15	1.		
21	1.	ሳ •		
22	1.	٠ ٢ 5		
13	1.	. 5		
24	1.	. 15		
25	1.	1.		
26	1.?	0.		
४७	1.2	. 25		
≥8	1.2	. 5		
29	1.2	.75		
, n	1.2	1.		
31	1.4	. 0		
12	1.4	. 25		
53 54	1.4 1.4	•5 •15		
,5 1 5	1.4	1.		
36	1.6	ı) .		
37	1.6	. 25		
38	1.6	• •		
30	1.6	./5		
4.0	1.6	1.		
41	1.8	0.		
42	1.8	. 25		
4.5	1.8	• 5		
4.4	1.8	./5		
45	1.8	1.		
46	2.n	₽.		
4 7 	2.8	• 25		
48	2.0	• >		

2.0

2.0

1.

41)

0.1

11.1

. 1

0.1

0.1

0.1

0.7

6.1

0.1

Nor REPRODUCIBLE

აწ

,5

.1 U

0.1	1) B	وڊ	34
6.1	1	29	15	7.4
6.1	1	20	311	35
0.1	1	51	37	36
0.1	1	. 31	,2	7
8.1	1	12	.58	3/
8.1	1	12	., 3	38
R.1	1	43	30	38
0.1	1	33	34	19
0.1	1	34	40	39
0.1·************************************	1	34	35	40
0.1	1	16	42	41
0.1	1	16	37	42
0.1	1	37	43	42
0.1	1	47	58	45
0.1	1	18	44	45
0.1	1	18	39	44
0.1	1	19	45	14
0.1	î	19.	40	45
0.1	ī	41	47	46
0.1	i	41	42	47
0.1	1	42	48	47
n . 1	1	42	4.5	48
0.1	í	4.5	49	48
0.1	1	4.5	44	19
r.1	1	44		
0.1	1	44	ე∏ ყ5	49 50
** • T	1	44	47	วเ

Butter John Tox

Program Output

NJTS. # 50	KH = ",	WRE = 0 MPL = 12 NMUH E 9 MKEY E 2 MLUMP = 0
j	۱۹	L R I L S essession and a session and a sess
	9	BOILCON BATTO MOUNT IN OF BILLIAN WATER
A I E	UNG . S MUHALUS	
7	0 - 10 B A B F B B	0.115.50E 08
J 0 1 M T	υ	<
	X C0081	ן ≻ ֿ
~		6-07-05 B B B B B B B B B B B B B B B B B B B
4 3	e- =	0.77.8HO
9 ~	n.25000	
6	90.22.0	0.519116
	3 7 5 4 6 4	8.25540 1. 01000
11	4.58809	
2 T		5.000±0 5.50±00
14	2.V00000	D.77686
46	4.7500#	
	66647+4	
	B.75480	01047.0
~ . = -	2 . 15400	
22	- T. B. B. C.	
24		0.21800 0.21800
25	1.04983	
26	1.28880	ຄ. 2-2-8 ເກ
e 0	1.28884	
6.0	1.78992	
37	20204	0.25.000
3.1	1.48#88	
4 60	1000 T	0.2544P
14.	1.79890	
3.7	1.00000	C.25088
10	1.08089	ν 7.79¢μ
4 4	30780 · ·	1.000A
ç	E 8 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
1	# d & & .	TOURSE TO THE TOUR TOUR TOUR TOUR TOUR TOUR TOUR TOUR
77		

					F. F						
		RAINED JOINT									
	ABOUT X ROTA	AT FACH UNREST									
7.40000 0.5000 7.40000 0.5000 7.40000 0.50000 2.40000 1.0000	DISPLACEMENT ROTATION 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	FOR EACH 2 DISPLACEMENT ND. NO.		111	15 17 18	10 20 21 22	25	2.9 3.8 3.1 5.7	53 54 56 57 57	30	45
46 7 7 7 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1	JOINT NO. Z DISK	COOPINATE NUMBERS FOR	0 = - 2 - 7 - 7					34 35 36 37			

THE PROPERTY OF THE PROPERTY O

10 10 10 10 10 10 10 10		2000	,		0 24747	0 3474746	4 Mark 4 Co. 12	F4 F4 F4	•
	64					36/1/200	201620-8	0 - YO L 5 4E	
1	e 4	1.100:	÷ ÷	~ ~	.27473E	0.27473E	0.02416E	0.96154E	
1	19	0.1800	42		.27473F	0.27473E	8.82418E	0.96154E	
10 C C C C C C C C C	c x	n.1000	¥; ₹		.27473F	0.27473E	0.82418E	8-96154E	7
1 1 1 1 1 1 1 1 1 1	7.0	0.1000	43		.27473E	8.2/475E	0.82418E	9.96154E	
	~ C	0.1000	* * *		.27473E .27473E	8.27473E U.27473E	8.82418E	0.96154E	3 3
1 1 1 1 1 1 1 1 1 1	,				`				
1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	3000	PE	0 X	L A P S 1	FFNES	ATRI			
1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1000						
1879/F 12 12 12 12 12 12 12 1	38184F 41532F	こっちゃんのフィル	1.6.3.80E 05	-0-24859E n5 -0-22117E n4	. 21634E 0	0.18752E 0	0.15114E	.28526E	
1.27056 N0.510416 N0.5104	-13656E	-0.23826F	4820BE	8.9557BE 84	.24870E B	# 15193E #	8.61424E	.14326E	
1.5779E 07 -0.51140F 06 0.1190AME 80 -1.28790E 85 -1.28730E 80 -1.28721E 81 -1.28731E 81 -1.28731E 82 -1.28731E 81 -1.2873	.23900E	-0.13544F	.24879E	-0.37753E 111	.18083E 0	.44329E 8	.26342E	.55807E	
15.575 F. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.			20400	37 07 48	240000	44476	236404	1000	
1.16075E 01 -0.00701E 01 -0.00701E 01 -0.11701E 01 -0.1011E 01 -0.	= =	143605	517816	1 - 381 5 F	12/136	6 4655.4F #	AASA1E &	21.1746	
1,0075F 13 -0.11701E 13 -0.1707E 12 0.2004E 13 0.2724E 13 0.2724E 14 0.20074E 15 0.2724E 16 0.2724E 17	: c'	0.60258F	966.18E		.1961BE	8.18710E 0	1.21515E 8	8-23586E	
1.2256 87 -8.92936 66 -8.62926 89 8.44956 69 6.52336 84 -8.44966 80 -8.667356 99 8.422496 99 8.43996 81 -8.42236 89 8.422496 89 8.43996 81 -8.22236 89 8.422326 89 8.42236 89 8.	C (E	-4.1791/E -6.18121E	4.71975E 4.28126E		./2275E	.32164E 0 .33249E 0	.68785E 9 .18464E 9	0.19415E 0.71597E	
1200RE 03 - 1300RE 04 - 1300RE 05 - 1400RE 04 - 1400CA 06 - 1400CA 09 - 1400RE 05 - 1220RE	m								
19709E 13 - 11209E 14 - 10-1550FE 14 - 1-22707E 14 - 1-23709E 17 - 1-21999E 14 - 1-21999E 18 - 1-22709E 17 - 1-22709E 18 - 1-227	8.15662E	-R.50563E		0.15932E 05	.52330E 8	8.4496/E	9.66763E	0.38249E 0	8.36398E
1.1764FF 33 -0.111361F 43 -0.16691E 60 0.18990E 81 0.19990E 82 0.27774F 62 0.2597E 62 0.16691E 83 -0.26776F 62 0.27774F 62 0.277774F 62 0.27774F 62 0.27774F 62 0.27777F 62 0.27774F 62 0.27777F 62 0.27774F 62 0.27777F 62 0.	0.188816 83	0.10659E		8-8-574E n4	.22327E #	6.45619E	0.27998E	0-21284E 0	9.24693E
14400E 17 - LEESBRE AG - LEEGBRE AG - LEEGBR	. 17640F n3	-0.11361E		0.1A993E #1	.19916E	.27754E	.23976€	2022010	30/2210
127059E N3 0.15796E N4 0.52943E N5 0.0404E N3 0.5596E N3 0.22059E	4 1.34986	-Uc.1558RF		b.24778E 05	-41272E 84	, 0	0.52917E 0	99684F 8	0-63327E 6
1.27460E n.) u.17524E n. u.17524E n. u.27527E n. u.27547E n. u.275	8-12591F RS	140041	4.53943E 05	-0-12791E 05	-67898E 83		0.34897E	8.22947E 9	379999-9
1.251860E 8.3 8.39595E 88 -8.18548.E 8. 8.2978.E 8. 8.2978.E 88.2180E 88.21805E 88.2180E 8	8.27859E 63	-8.17534E	#.11471E #3	12-34541E 05	. 80020F 03		8.39988E 9	0.318/8E 0	8-26678E
5.446315 66 8.18169E 8488492E 84 8-23129E 85 8-128635 856.33881E 85 8-2482E 87 8-24635E 87 8-24636E 87 8-24	. 8.21960E #3	9.39585E	-8-18563E 81	8-59/83E 81	.12843E 82				
	5.48833E	9.18169E			.12603E 05		0.338818	.21288E	8.67875E
2.35710F N3 -0.59084E N2 -0.63344E N3 -0.44714E N3 -0.444315E N3 -0.24346E N3 -0.2575E N3 -0.4575E N3 -0.47714E N3 -0.444315E N3 -0.24346E N3 -0.4268E N3 -0.463345E N3 -0.444315E N3 -0.45846E N3 -0.463345E N3 -0.444315E N3 -0.45846E N3 -0.46436E N3 -0.44436E N3 -0.46436E N3 -0.	ı	9.68639E			.18044F 03		0.15234E	0.24092E	0.14530E
1.8661E N8 -8.59884E NR N.18758E NI -0.46714E NI 0.12213E 02 1.3661E N8 -8.59884E NR N.18758E NI -0.46714E NI 0.12213E 02 1.3647E NI -0.57658E NI -0.4677E NI -0.4677E NI -0.45697E NI -0.45697E NI -0.49697E NI -0.49697A NI -0.4969A NI -0	37510F	# 26762E			.44313F 00		0.79428E	8-19735F	8 - 1047/n 8 - 74445F
6.35117F 96 -0.19090E 0.0 0.59973E 0.0 -0.1676E 0.0 -0.18485E 0.0 -0.57658E 0.0 0.36510E 0.0 -0.17412E 1.35177E 0.0 -0.19090E 0.0 0.35677E 0.0 0.26436E 0.0 -0.18485E 0.0 -0.13651E 0.0 -0.17405 0.0 0.26436E 0.0 -0.19607E 0.0 -0.17209E 0.0 -	186816	-8.59084E		-46714E	.12213E 02				
R.36/4/E m3 -0.42970E m3 e.56477E m5 -0.41255E 03 0.26396E m4 -0.45651E m2 -0.19697E m5 -0.30161E m5 -0.48037E m3 -0.42970E m3 e.572970E m3 e.48649E m3 e.48640E m3 e.48649E m3 e.48640E m3 e.25341E m3 e.48640E m3 e.52550E m3 e.524514E m3 e.48660E e3 e.46670E m3 e.524516E m3 e.52551E e3 e.48650E e3 e.48670E m3 e.524516E m3 e.52551E e3 e.48670E m3 e.524516E m3 e.52551E e3 e.61900E m3 e.524516E m3 e.524516E m3 e.52551E e3 e.61900E m3 e.524516 m3 e.524516E m3 e.52551E e3 e.61900E m3 e.524516 m3 e.446010 m3	6 1.35117E	-8.19896			./1676E 84	9.18485E	8.57658E	.36518E 0	8-17#12E 8
8.1364/F m3 -0.629/RE m3 -0.17209E m3 0.05374E m3 0.05379E m3 -0.1849E m2 -0.1849E m3 -0.1		0.68176F			.26388E 84	0.45851E	# . 19697E	.30161E 9	8-48037E B
1.15(199E n.3 - n.68436E n.2 - n.18345E n.2 - n.18345E n.3 - n.13975E n.4 - n.58468E n.3 - n.13465E n.3 - n.13468E n.3 - n.13465E n.3 - n.13468E n.3 - n.134	1364/E	-6.4840.SF	1451 BF		. 65.374F D	JORBHE .		41/259E 0	0.29523E 0
7 8.11572E 87 -8.58778E 86 8.19554E 86 -8.36196E 85 8.31551E 85 -8.43975E 86 -8.58468E 85 8.48826E 85 -8.35176E 8.81272E 83 8.14187F 96 8.51986E 85 8.48734E 83 -8.78962E 83 -8.448143E 85 -8.24513E 85 -8.35176E 8.23341E 83 8.48899F 83 8.46276E 84 8.82288E 84 0.16679E 84 8.12621E C3 -0.16313E 83 -8.25258E 84 8.244361E 8.11618E 83 -8.18666E 83 8.46276E 82 8.54608E 83 0.65337E 83 8.27573E 85 8.58824E 82 -8.61988E 81 -8.86181E 8.11618E 83 -8.52521E 87 -4.15371E 82	16199E	-8.68436F	.18345E						• 3//63//6
8.11572E 87 -8.58775E 86 8.1974E 80 -8.30190E 85 9.31551E 85 -8.43575E 80 -8.55468E 85 8.48826E 85 -8.13465E 85 8.2341E 83 -8.43544E 83 -8.4344E 83 -8.24514E 85 -8.15414E 83 -8.5251E 62 -8.61988E 81 -8.65141E 83 -8.5251E 67 -8.15414E 83 -8.5251E 67 -8.15414E 83 -8.5251E 67 -8.15414E 83 -8.5251E 67 -8.61988E 81 -8.66181E 84 -8.27571E 83 -8.52571E 87 -8.66181E 87 -8.61988E 81 -8.66181E 87 -8.61988E 81 -8.66181E 87 -8.61988E 81 -8.61988E 81 -8.66181E 87 -8.61988E 81 -8.66181E 87 -8.61988E 81 -8.61988E 81 -8.66181E 87 -8.61888E 81 -8.66181E 87 -8.61988E 81 -8.66181E 87 -8.61888E 81 -8.66181E 87 -8.66181E 8	7		- 1	- 1					
D-23341F #3		-0.587/8E			.31551E 05 .82513E 03	.439/5E .70962E	8.58468E 8	0.48826E	0.13465E
8-11616E 83 -6-52521E 67 -6-15421E 82 8-11616E 83 -6-52521E 67 -6-15421E 82 8-11616E 83 -6-52521E 67 -6-15421E 82	-23341E	# 43898F	.9328BE		.16679E 84	12621E	0.16313E	8.23250E	1.24361E
The property of the property o	-11616E	-0.52521E	15371E		20000	201013			
THE PERSON OF TH	6								

0.11651F 07 0.13192F 05 05 05 05 05 05 05 05 05 05 05 05 05								
2 c c c c c c c c c c c c c c c c c c c	1.19591F n6	=		.45631F B				
2	1 - 14.546F nn	"-5.17RZE		0.1105HE A				
2 0 4 2 4	0.21363F n2	8.95348F 02	0.47031E n3	0./3569E n3	-8.89119F 88	-8.34250E 83	-8.17623E 84	-0.20078E 04
0 e c c c c c c c c c c c c c c c c c c					;			
# 5 G	0.18220E n4	.89/A1E A	.22910€	.180035	-4-191056 86			-8.65248E 84
2 = =	5892BE	=	.24285E	.17/25E	8.93778E 63			8.39184E
	0.28589E 42	-4-68175E 82	0.28069E 83	-0.27491E 00	0.18305E 01	#.197/7E 83 -#.53933E #1	-9-11564E 84	-0.47/56E 02
;								
136 84 0	0 - 18366F n5	1 - 3 - 3 - 4 - 4 - 4 - 4 - 4 - 4 - 4 - 4	-0.53393E 04	0.445915 84	-8.52396F 83	-8.238735 65	0.28822F	-0.47407E 84
	-26396E	.69/33E A	0.94662E n	.27286F	W.17648E	6.46339E	.18418E 8	1.29126E
*	.89578E	.53530E A	.295&6E 0	.56/42E			.54648E	
'	1.58686F AK	.19565E	0.34275E D	.25794F A				8-14273E
7.0	8.17865E no	8.06358E 85	-8.56886E #4	8-18327E 04	-0.35323E 03	-0.45737E 05	-0-21782E 65	-6.48676E 64
5.5	7.10442E n3	.146A5E	E C:	-10322E 8				-6-567605 04
1 13 n 11448E n 3	1 ~	9 2 1	2 10 10 10 10 10 10 10 10 10 10 10 10 10	474050	77 69 16	24.241885.85	170800	1 4
12	300	-	.6.676E B	9	14518E	-6.44585E 85	-8-22416E 65	-8.48458E 84
55	8.96858E n.3	n.98912F 84	0.80463E 04	0.17208E 84	8.90581E 02	-6.16/25E 83	-22836E 8	•
		- 1						
7	9		0.29482E					
8.16953E 85 8.44786F 83	9.11584F 44	6.63876E 83	-8.21955E 83 8.48173E 84	-8.23802F 62	0.73612E 84	-8-45/39E 85	-8.25238E 85	0.144886 80 -8.25/78F 84
16	32		0.31016E n3					
	0 37181	u u	224BeE	20000	245000	101405	203736	74114
		-4-26882E 82	0.33247E 63	-0.25252F 03	6.16419E 84	-6-19228F 85	-0-28826F 82	8.79496F 81
R 3	1.0855/E 4.5	6.48/38E	-0.24177E ne	.27442E		.25152E	0.11996E	
8.1	-18294E		.20535E					
	177505 0	00000	24766	4 70064	2000	1500.75	26.48	10277
•	11//200	20.00	10.3.38/2E	-0/850E		8./521/E	. 5048DE	36//wr-
45542F R3 -	4.15745F 45	# . Y . O . O . O . O . O . O . O . O . O	0.15531F R5	0.274726.84	10.2225 US	6. 1751 AFF 59	-8-339/6E 85	
20	.14396F	-U.388R6E 01						
2	47 H 59F 86		4485 0	454075 0	. 40852E A	A1088E	342316	20 30C cat a-
7			2005	24819F B	14715F B	127707E	27.47.25	41/7/6
i	-0.78641F A	9-17-581E 05	0.94540E 64	0.154635 94	8.36639E 82	0./8415E #2	-0.19819E 04	-8-17693E 84
· ~			•	1				
18 0.15170F 07 -0	1.47894E 06	8.91262E 85	0.17044E 05	B.97879F 04	-64886F		2457%E	410315
5	.71683E	0.82553E 95	0.42250E n3	49399E n	-0.12517E 94	-0.56935E 05	-0.28268E \$5	.55362E
2 5		-1177/	10100	. 19013E #	.3553/E		-19422E	-8.16555E 84

CAS CASS

والمائدة المتكاف والمتكافئة المسابل المحافظ

										•	-		
-0.92940E 04 0.11454E 03 -0.19015E 04	-8.28931E 82	-6.19069E B5	-8.11892E 85 -8.37416E 84	-0.10431E 04	-8.74527E 84 8.36518E/62	-8-69/98E 84	0.19#41E 85	-0.12287E 85 '-	-0.23.11E 04	8.74834E 84	8.56219E 84	8.21252E 85	0.13J9JE 05
0.21527E 03 -0.23954E 05 -0.18084E 04	U.15244E 84 -8.19989E 82 -8.10961E 84	U.46486E 85	0-44776E 85 -9-27236E 85	0.26743E 95 0.27873E 85	-6-16237E 65 -8-29281E 85	6.18129E 64 -	0.47192E 05 -	#.44361E B5	0.27987E 85 -	0.65718E 01 -	9.83454E 838	0.55272E 65 -8	8.46766E 05 -
0 -0.38395E 05 4 -0.58341E 05 2 -0.12485E 03	-0.15648E 03 -9.25035E 05	-0.10124E 86 -0.31322E 85 0.78427E 02	-0.57779E 05	-0.87107E 85 -8.57525E 85	-6.49934E 05	0.68782E 02 -0.26529E 05	-8.17217E 86	-8.88415E 85	-0.84571E 05 -0.39852E 05	-8.56982E 85 -0.37912E 85	0.72563E 02 -0.17921E 05	-8.79865E 85	-0.61348E 65
4 -0./0580E 0 3 -0.14596E 0 1 0.6343/E 0	5 -4.31273E 06 3 0.20600E 84 2 -0.81453E 02	4 -8.33408E 86 4 -8.66918E 83 4 -0.13827E 83	29514E 86 3 9.14584E 86	1 -U.71783E 06 5 -8.59745E 63	3 -8.7614/E 86 3 -8.55328E 83	-8.34249E 86	-0.33861E 86	-0.75735E 00 8.30953E 64	-8.72112E 86 8.29496E 84	-8.76362E 86 8.28138E 84	-8.34247E 86 8.29834E 84	-8.28871E 86 -8.15718E 84	-8.71869E 86
12 0.48071F 0	05 U.31808E U 03 -0.49370E D	5 0.67948E 0 5 0.67201E 0 4 0.11695E 0	0.53124E 0 4 0.14320E 0 4 0.6204E 0	5 0.17646E 04 4 0.13896E 05 4 0.11831E 94	5 0.58968E 8	5 0.26674E 05 3 -0.74769E 83	5 8.68787E 84 5 8.73288E 84	2 R.23472E 82	0.40353E 84	0.28472E R4 -8.97135E 02	6.27827E 85	0.68244E 84 0.84186E 84	0.22909E 04
04 11.40206E 01.09	05 U.703836 D 02 U.43659E D 04 -0.41870E D	05 -8.3348E 0 86 -8.10324E 0 84 0.75563E 0	0 -4.31335F 5 -0.82298E 4 8.66376E	5 0.14/52E n5 5 0.3n311E n4	94 8.44/77E 0 85 8.66871E 8 84 8.65151E 8	85 8.31548E 05 82 8.50601E 05	5 -0.34144E 6 6 -0.13835E n	6 -8.32482E 85 5 -0.47039E 64	5 U.15/94E 05 5 D.14667E 94	0 0.45171E 05	8.31799E 05	-0.33949E n5	-0.37177E n5 -0.87595E n4
06 -0.85270E 006 0.71001F	14 -1.12/79E (16 -1.25765E (19 -1.25765E (19 -19 -19 -19 -19 -19 -19 -19 -19 -19 -	06 0-10/87510E 010 0-10/86E 0	06 1.16462E 0	16 U-83/57E B 16 H-879A2E B 83 H-85995E B	R6 -++-676.38E # R6 9-75124E # R3 8-84278E #	4 -0.14299E 6 -0.59953E 3 0.45441E	96 8.18188E 8	#6 8-17578E 66	1.785957E 8	6 -4.86617E 84	4 -4.14474E 95 6 -8.57286E 92	A 4.86877E 85	4.174R1E B6
07 -0.19765F 0.25205F 0.31030F	06 0.257986 004 0.102866 00 00 00 00 00 00 00 00 00 00 00 00 0	06 -0.18136F 0 04 0.11561F 0 03 -0.41527E	07 -0.46403E 0 04 0.24235E 0 03 -0.46/76E 0	07 -0.46621E 0 0.23413E 0 83 0.21974E 0	85 P.24722E R	96 8.26711E n 84 8.11854E n 83 -8.95214E n	6 -0.19896F 4 = 10284E 5 -6.25686F	87 -8.47698E 0.84 8.23124F 9.	87 -9.48981E 36	17 -8.19341F 46	6 8.27924E 8	6 -8.18587E s6	7 -8.48253E 4 5 4.15636F 86
0-13445E 0-16466F -0-51/98E	20 0.63435E 0.96359E	21 0.55052F 0.37236F	22 1-15456E 1-74344E	ROW 23 0-17318F n 6-14903F n -6-13429E a	80W 24 0.15559E n 0.13682E 0	1 8.5661E 8 8.5661E 8 8.5855E 8	804 26 8.55249E 8 8.39185E 8	ROW 27 8.15767E 8 -9.54317E 8	28 0.17711E 0.11189E	8.15893E 8.07974E 0.07974E 0.07974E	9.57319E #6	#0# 31 #53872E #0	9.15559E 07 -0.11499E 05

	-8-59496E 84	-8.54193E 84	-0.2338/E 05	-0.11016E 05											8.32322E-02	\$ 25917E-84	6.22937E-04	8.27659E-05	6.11489E-04	8.2716/E-84.	40-3587.88.4	9-11768E-04	0.17670E-84
	-6.14952E 83	8-14828E 94	W-48167E B5	0.38192E 05	0.16593E 65										8.195149E-85	0.38352E-04	0.271406-04	8.4123/E-05	0.13298E-04 0.21899E-04	0.24843E-84	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	8-12125E-44	9-17817E-84
	-8.28926E 65	9.52442E 92	8.44879E 84	6.38498E 65	8.55173E 85	6.44781E 05							1 !	×	0.23989E-84	8 - 10203E-94	0.31385E-04	8.58253E-85	0.15845E-04 0.22859E-04	6.18/69E-04 8.26634E-04	A 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	9 . 12324E - 04	0-17895E-64
	-6.71017E 06	-0.32243E 86	-8.12768E 85	-8.5927/E 86	-0.40788E 96	-6.48488E 00	-0.18737E 96							* * * * * * * * * * * * * * * * * * *	0.15767E-84 6.38731E-05	0.141055-84	0.35686E-04	4./459/E-05	0.16/38E-84 8.12/08E-84	0.28564E-04	1	8 12328E-84	
	0.1950&E 05	8.38178E 85	#.59883E B4	8.65615E #5	0.92647E 05	6.958856 85	9.83852E 85	0.49778E 84							0.55368E-07	0.18238E-04	0.40824E-04	U.85c99E-05	0.145065-04	0.27353F-04 0.30203F-04	0.5486AE=05	0.122226-04	0.1462/E-04
-0.13831F 04	8.43493E 85	8.29938E N5	-0.29837E n5	-0.33274E 85	8.54714E 64	8.57444E n4	0.28956E 04	-6.21053E A5	-0.323005 05					× ×	U-10445E-04	9.27633E-04	U-17/06E-04	#.5.5092E-46	0.174758-05	U-31979E-04	408525-05	• H111/E-05	B.14887F-04
0.357435 02	-0.81659E 04	-0.13566	0.77.5A1E 05	n.17814E 86	0.90>35E 05	-0.53837E 54	-6.30154E 84	8.58978E 85	9-140ABE 06	8.85332E 85					1-16145E-05 0-14958E-04	0-27163E-84	0.238926-04	1.112186-05	1.95479E=05	4.25982E-84 8.21486E-84	6-16421E-45	1.87131E-85	1585,31-84
	-0.19745F #6 0.15235F #6	0.719446 05	-0.21256E An	-0.54129E 86	-9.25172E ac	-6.24518E n6	. D. 94/42E n.3	-0.11884E n6	-6.34789E no	-8.38273F 86	-0.17597F an			2 2 3 3	4.28012E-14		-	くり-36番がな/*ロ	F-11764E-94	n.15/356-n4 n.235986-n4	# 19422E=#4	ļ	B. 15384FINA
0.61312E. n3.	ROW 34 0.15/46E 0/ 0.17563E 04	80W 35 0.56827E n6 0.13498E 03	80W 36 8.39789E 86 8.59849E 84	ROW 37 0.13551E 07	ROM 38 0.15796E 87	80W 39	POW 48 0.48428E 06	48k 41 0.18859F 86	RGW 47	RON 4.1 0.68976F 96	#OH 44 0.55299E Ab	80K 45 6.17363E 66		2 L 2 C L 2	8.92986E-05	8-19537E-84	4.32436E-84	RDW 2 2 112F-85	#-12K#4E-84	8-17532F-84	ROW 1	4.64074E-05	8-15-84 PF-84

16346E-05	8412E-84	19846-04		40-01-80	10-282E-05	11/23E-85	21291E-84		よりの出版。 よった。 おった	0.051E-0.0	つかし レートーマー	34496	5.566E-04	1/186-05		7/58E-84	0464E-84	0477E=03	1	6/30E-64	0.46E-03		893VE-04	9426E-04		3399E-84	4212E-03		7126E-84 4345E-83	11156-03		26716-03	6.322E=6.3
.53947E-05	.13201E-04	-20678E-04		108705-8-8	.56/31E-05	.17/1#E~B4	.1855#E~84		j	1			ì	1		.29090E-04	.81482E-64	- 1 85 6 CB - 8 C	1566%5-84	.617656-84	.96528E-84		.31196E-84	.84732E-84	Į	-					į	8.89532E-84 8.1	
				20.00000	U-14176E-64	U.15003E-04	G-16G43E-04		0.47587E-84	6.53946E-04		A . 34182E-84	\$ -67879E-\$4	8.86389E-84		0.31462E-84	0.823566-64	0.106536-93	4.00 5.00 5.00	8-57996E-64	0.92567E-84		0.24867E-04	8.75938E-88		#.12556E-#3 #.28797E-#3	0.14678E*03 0.22573E-03		16624E-03 16772E-03	23538E-03 21455E-63		8.93712E-84 8.14925E-83	・エ・アインロール
				11.451075=04	0.11269E-64	0.173736-04	U-15483E-04		8.61896E-84 0.29884F-84	9.65/53E-#4		0 - 40 E 5 EF - E 4	6./3986E-84	40-Baecce - 9		9.32999E-84	8.83136E-84	6.91842E-64	0.34124F-84	0.54079E-84	6. 8060KF-84		8.45119E-84	0.67299E-84		0.15018E-03 0.91382E-04	8.16841E-83 8.24889E-83	1 - 1 - 20 - 1	#.11cule=#3	0.15985E-03 0.22075E-03		8.15156E+84	
0.49978E-05	0.13185E-04	0.20850E-04	.244, BE	H . 58064E - 05	0.858R2E-05	0.9849/E-05	U.23U94E-04		0.6250/E-05 0.40255F-04	0.17979E-04	1	8.45279E-04	0.570596-84	8.985858-84		0.33162E+84	8.67612E-84	8-96114E=64								8.36400E-64 8.11170E-83	0.19059E-03 9.27000E-03	n. 19308E=#4	0.10394E-03	0.17119E-#3 0.23893E-03		8.98653E-84 0.15357E-83	1
0.348178-0	0.15967E-0	11.19552E-A	0.23116-0	11.345795-05	0.62108F-05	11./4038E=43	0.20463E-04		0.21628E-04	1.906775-64	0.918416-04	.10119E-0	.43130E-0	.18485E-8		.32811E-n	.6A839E-8	• VOL 1 OE = E	u.26471E-n4	8.46146E-84	0.89835E-04		N.26712E-04	0.93936E-84		#.50365E-04 0.13245E-03	0.21339E-63 8.20373E-63	B. 45H775-84	8-21579E-43	0.25189E-03		0.11641E-03	, , , , , , , , , ,
4.271565-05	# -14605E-P4	11.18246E-04	4.21401E-94	1.198458-05	1.48239E-05	1.158415-04	11-17HK0E-04			11.18363E-83	1104116-05	# - 1488E - 84	0.49.508E-04	2.17.10E-03		4.14689F-84 4.45744F-04	# . 69801E-84	**************************************	8.23487E-84	#.58937E-#4	#.#5H95E-04		n.24879E-04 8.52653E-84	N.75558E-84 2.85211E-04		n.15543E-04	3.23655E-03	8.55.31 8.45.31	0-12798E-83	#.19558E-#3		4.12881E-93	
0.232051-05	0.133926-04	0.169348-04	.2049UE	11.78.5485-06	0.11949F-04	0.14216F-04	# 15297E-04		1.27834F-64 1.78254E-04	0.416675-04	0.11652E-n3	8-188245-04	0.55549F-44 8.86391F-64	# . 73584F - 94		0.17288F-14	D.78716E-A4	6-18273E-03	0.21898F-84	8-47482F-84 H-64667F-84	6.8194eE-64	i				0.85748E-84	8-10788E-83	B.A5782F-44	8-1300E-83	1.18677E-43			
	0.12072E-04	0.1562UF-04		ROW 5	0.878338-05	0 114405-04	0.127648-04	ROW 6	0.165926-04	. 52952E-6	12909E-0	ROW 7	0.61456F-84	0.799746-04		8-19296F-84	0.71401E-04	0.1037RE-#3	20H 9	0.43576F-84	0.77979F-84 0.18439E-83	ROW 10	8.38394E-84 8.47685E-84	8.68349E-84	RON 41	8.11874F-B3	0.21938E-83	A.76472F-84	0.15155E-#3	#.19896E-#3	11	0.64/69E-04	
	29675F-05 0.23205[-05 0.27156E-05 0.37817E-05 0.49978E-05 0.62435E-05 0.67818E-05 0.53947E-05	1.29675F-75 10-23205f-75 11-27156E-75 0.34817E-75 0.49978E-75 0.62435E-75 0.67818E-75 0.53947E-05 0.79819E-75 0.926701E-75 0.13592E-74 0.15592E-74 0.15592E-759E-7592E-7592E-7592E-7592E-759E-7592E-759E-7592E-759E-759E-759E-	1.29475F-75 1.27205f-75 1.27156E-75 0.34817E-75 0.49978E-75 0.62435E-75 0.67814E-75 0.53947E-05 0.79819E-75 0.92610E-75 0.53947E-04 0.79819E-75 0.92610E-75 0.33501E-04 0.13501E-04 0.13501E-70 0.15817E-74 0.13185E-74 0.14501E-74 0.15817E-74 0.1721E-74 0.15620E-74 0.15817E-74 0.15817E-74 0.15818E-74 0.15818E-74 0.15620E-74 0.18959E-84 0.19367E-74 0.20678E-04 0.18959E-84 0.19367E-04 0.20678E-74 0.18959E-84 0.19367E-84 0.20678E-84 0.18959E-84 0.19367E-84 0.20678E-84 0.18959E-84 0.19367E-84 0.20678E-84 0.18959E-84 0.20678E-84 0.20678E-84 0.18959E-84 0.20678E-84 0.20678E-84 0.18959E-84 0.20678E-84 0.20678E-84 0.18959E-84 0.20678E-84 0.18959E-84 0.20678E-84 0.20678E-84 0.20678E-84 0.18959E-84 0.20678E-84 0.20678E-84 0.18959E-84 0.20678E-84 0.20678E-84 0.18959E-84 0.20678E-84 0.20678E-84 0.18959E-84 0.20678E-84 0.18959E-84 0.18959	### ##################################	1.29675F-75 1.297205f-75 1.27156E-75 0.34817E-75 0.49978F-75 0.62435E-75 0.67818E-75 0.53947E-75 0.79819E-75 0.92670F-76 0.13592E-76 0.13592E-76 0.15819E-76 0.17121E-76 0.15819E-76 0.15819E-76 0.17121E-76 0.15819E-76 0.15819E-76 0.17121E-76 0.15819E-76 0.18819E-76 0.17121E-76 0.18819E-76 0.19819E-76 0.17121E-76 0.28819E-76 0.18819E-76 0.19845E-76 0.28111E-76 0.28448E-76 0.25721E-76 0.29721E-76 0.297	1.29675E-05 0.23205[-05 0.27156E-05 0.34817E-05 0.49978E-05 0.62435E-05 0.67818E-05 0.53947E-05 0.79819E-05 0.23205E-04 0.13591E-04 0.13591E-04 0.13591E-04 0.15817E-04 0.15817E-04 0.17121E-04 0.15817E-04 0.17121E-04 0.17121E-04 0.15817E-04 0.17121E-04 0.27121E-04 0.17121E-04 0.27121E-04 0.27121E-05 0.2712		0.29475F-05 0.24617E-05 0.49978F-05 0.62435E-05 0.674148E-05 0.53947E-05 0.19619E-05 0.90487E-05 0.90487E-05 0.19978E-05 0.13501E-04 0.13501E-04 0.15017E-05 0.15016E-05 0.1501E-04 0.14501E-04 0.15817E-04 0.17121E-04 0.15017E-04 0.15016E-04 0.15016E-04 0.14501E-04 0.17121E-04 0.17121E-04 0.15017E-04 0.15016E-04 0.15016E-04 0.15016E-04 0.15016E-04 0.17121E-04 0.15620E-04 0.15817E-04 0.15817E-04 0.17121E-04 0.17121E-04 0.23786E-04 0.16946E-04 0.24478E-04 0.180578E-04 0.20678E-04 0.23786E-04 0.24478E-04 0.24478E-04 0.21002E-05 0.36678E-05 0.86783E-05 0.05108E-05 0.05108E-05 0.09840E-05 0.13485E-05 0.13485E-06 0.11669E-04 0.12485E-06 0.10465E-05 0.10465E-06 0.13485E-06 0.13485E-06		1.29675F-75 11.27207F-75 11.27156E-75 0.35817E-75 0.49978E-75 0.62435E-75 0.67818E-75 0.53947E-95 0.79819E-75 11.29670E-75 11.2725E-76 11.	10.20075F-05	0.29675F-05 U.92305F-05 U.37166E-05 U.34817E-05 U.49970E-05 U.22435E-05 U.6276E-09 U.32976E-09 U.3267E-09 U.3267E-09 U.3267E-09 U.3267E-09 U.32676E-09 U.32676E-09 U.32676E-09 U.32676E-09 U.32676E-09 U.326772E-09 U.326772	0.29675F-65 0.23205F-65 4.27156E-65 0.36817E-65 0.49978E-65 0.62435E-65 0.67818E-65 0.53947E-65 0.29819E-65 0.23619E-65 0.2361		0.15620F=05	0.25075F-05	1.29675F-05	1.20075F-05	1.2947F= 15	1.278.75E-75 1.278.05 -75 1.27156E-75 1.288.75E-75 1.289.75E-75 1.289	10.0075F=10	1,000,500 1,000,000 1,00	1,200.5E-07	2.7015/2-01	296782=0 0.72782=0 0.7278=0 0.7478=0 0.7478=0 0.7478=0 0.7278=0	200762-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0	17.5709E=14	1.2710/20-14	1.2720E-14	1,000,750,700 1,000,750,700 1,000,750,70 1,	1.27978E=14	1, 20,716 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	1.000 1.00

8.47677E-03	8.45412E-#3	9-49491E-63	8.46772E-03	8.44126E-63	8.41681E	1.40174E-83	0.43171F-83	8.48433E-83
0.37800E.B3	8.35332E-8.	\$.32979E-83	0.36783E-03	0.34027E-03	B-31453E-83	4.20188E-8.5.	0.26801E-43	25 30118E-83
	8-47749E-83	0.46848E-83	0.45898E-03	0-44538E-03	.43986E	0.41887E-03	8.48219F-83	.51551E-#3
0.483275-83	20-319575-0	8.34361E-63	8 - 33595E- 83	0.326H6E-03	0.317346-03	8.38R19E-05	• • •	24 -26928F-#3
8.45861E-83	0-40/10E-03	00-1/201-02	20-37176	0.52775E-03	.53607E	54364	#.55115E-43	R.55791E-03
8.41942E-53 -	\$6-39802E-63	8-32958E-83	U. 33786E-03	0.344295-03	U.34993E-03	1.251705-03	4.26893E-113	28 -26968E-43 -41378E-83
\$.46392E-93	6.48905E-03	6.51461E-63	8.53863E-83 6.59996E-83	0.53512E-03	8.25.85 8.25.83E-85	#-54537E-#3	0.61815E-n3	0.43887E-63
8.29/96E-83	8.32192E-83.	8.34869E-83	8.37113E.83	0.39455E-AJ	-22976	#.25289E-63	-27541E	22 29899E-83
		0.49122E-03	0.53299E-03	0.57508E-03	.019/1E-	• 00 • 1 /E-B	-410/35-#	
8-31456E-83 8-58237E-83	6.35451E-83. 6.54621E-83	9.39769E-83	8.44312E-83 0.34096E-83	12.5	0.24394E-03	28454	8.52898F-83 8.58726F-83	4.36582E-83 4.27579E-83
	4-32963E-03	8-37767E-E3	003226/1-63	60-4/946664	**************************************	TYTER XX		
8.22/62E-84	6.26/38E-6.3 6.27173E-6.3	6.1881/E-83	8.22862E-83	8.20567E-83	-18449E	# - 16246E - 9.3	8-14764E-53 8-27206E-63	8.18248E-83 0.24935E-83
8.39148E-03	0.36384E-05	8.37457E-83	8.3668JE-83	8.35749E-83	H.54682E-83	0.33/72E-83	8.32928E-83	.378/4E-83
6.23824E-85	0.22198E-03	8.28849E-83	9.29189E-83	0.19371E-03 9.27546E-03	#.1R522E-#3	8.17778E-83	.16862E-#	19 0.15600E-03 9.23983E-03
0.38574E-03	76-306867 · 0	8.39577E-83	8.48833E-03	6.33256E-03	1		8-348146-83	.35268E-83
200 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	6-12989/E-63	8-19988E-83	0-20161E-03	6.20055m-04	4.28875E-85 9.23698E-85	U.14147E-03 H-24312E-03	# . 14864E = 0.5	9.15487E-43
0.404775-83	8 - 42574E-05.	8.44258E-83	8.31/33E-83	u. 33634E-63	18646.	8.57431E-86	8.35667E-83	. 38572E-#3
8.17896E-85	24.1882/984.48 4.48-88-84	0.20736E-03	0.22585E-03 6.3435E-03	8.24329E-03	W.12468E-43	11.2566E-11.3	8.27544E-03	8.17565E-83
8-4516BE-#3	0.48615E-04	20-37882778	*.33536E-#3	0.J6665E-83	0.448756-63	11.4351.11E-11.3	- 7	41/59E-03
00-360464.000000000000000000000000000000000	8.2123/E-85 8.549/E-85	8 - 24597E-85 8 - 38449E-83	8.28194E-83 2.28141E-83	6 - 19899E-93	8.13586E-83	0.16896E-96	6 - 19358E-83 8 - 53554F-83	8.22717E-85 8.15378E-85
8 - 17.12.8E-03	8.2889JE-83	8.19339E-03	8.17839E-63	8.16315E-RS	# 14658E-#3	4-18618E-63	8 - 1 / 8 5 2 E - 8 5 8 - 2 8 8 9 4 E - 8 5	8.177; NE-W3
0 0 1 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8-94847E-04	0.81567E-84 8.16342E-83	8.11714E-83 0.14754E-83	0.18055E-83 \$.13215E-83	8.85558E-84 8.11/54E-83	9.18352E-03	8-14852F-04	.12434E-B3
		,	1 1				40.705	15.048985.04
8.28487E-85	8.1976JE-83	0.19115E-03	9-18462E-83	0.17868E-83 0.22988E-63	8.17912E-#3 8.27254E-#3	H-17274E-B3.	6.16626E-#3	#.15972E-#5
				、アルー出すにすりて、の	.12834	H-12489E-83	-	-11660F-03

	7. 2	22	9)	o -5									,	•				
	0.75841F-A	8.47583E-83	P. 6.0386/E-6.3	6.591956-63	8.58276E-83	B.74482E-93	6.71285E-03	E.92295E-03	0.86441E-03	8.85415E-85	6.18563E-82	0-10165E-02		,				
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0-80503E-03	0.50049E-03	9 - 28 3 3 4 E - 8 3	Ce-3662644	8.25418E-83	8.C2472E-83	0.759896-85	0.74118E-85	20-756-84-84-84-84-84-84-84-84-84-84-84-84-84-	8.82797E-03	0.11292E-82	#.18626E-82	0-10477E-02		٠			١
0.6827UF=8.5	0.86858E-03	8-53786E-65 8-76455E-05	W. 51465F-83	9.52852E-83	8.52711E-83	3.36964E-03	0.78969E-03	#.75861E-93	8.77176E-83	6.78829E-83	9-12#B1E-02	9-111966-62	8.18696E-82	0.18824E-02				
0.65858E-03	8.54848E-03	0.5690JE-63 0./9570E-03	#.52558F-8A	8-52044E-83	8.25549E-83	8 - 93731E - 8 4 6 - 81389E - 8 5	6.82826E-83	8./7341E-03	8.76388E-03	8.89347E-65	9.12907E-02	0.11582E-42	0.18981E-62	8.18754E-82	**************************************			
0.35755E-n3		8.29902E-U3 0.57284F-63	6.7.32826-83 6.7.3468-83	8.51090E-03	8.52466E-03	8-56125E-03 n-87354E-03	4.86533E-83	8.78634E-R3	4.75432E-03	8.76739E-63	0.62891E-83	0.12038E-#2	8-11867E-02	8-19656E-82	0.10/76E-02	0.11663E-02		
5-40187E-05		8-58147E-83	8-24-45-9E-03	8-24891E-85	B.40427E-83	#461569E-83	#_\$837E-#5	9-7983E-93 8-86283E-93	B-/4420E-03	8./3352E-83	0.89461E-0.3	#a45799E-R3	D-11219E-02	G-10550E-02	P.18588E-R2	P-12423E-92	F-11969E-82	
11 - 44 B5 3E = 0 3		0.41047E-03 0.63530E-03	11.09848E-11.3	0.49147E-63	0.61.55E-83	U-67316E-83	n.62447E-113	4.61699E-83	9.73476E-43	W.78271E-83	#.96266E-#3	R.98278E-A3	0.88956E-R3	9.18458E-62	4-18646E-62	8.13249E-02	0-12584E-82	8.123246-62
1.4900/E-03	-	U. 44843E-03	R. 419781 - 113	0-43465E-83	8-44145E-83. R-04518E-83	9.73474E-n3	0.66174F-03	8.63377E-83 8.89762E-83	9.64845F-43	0.67493E-83 k.97165F-83	# . 1 & 368E - # 2	P.94784E-83	0.01148F-4.5	8.97498E-n3	8.97392F-83	8.14162E-n2	0-13873E-82	8+12631E-92-
0.55423E-03	0-607716-83	ROW 27 0.47059E-63 8.69763F-63 8.66495F-63	308	ROW 20 0.47859E-83 h.68289F-83	اغذا	#0# 31 #.##123E-#3 #.6#622E-#3	#0# 37 9.64015E193 8.60477F193	300 00 00 00 00 00 00 00 00 00 00 00 00	### 34 8.64252E-83 8.87489E-83	RGW 35 0.68786F-83 8.88516E-83	ROW 36 9-11167E-82 8-98879E-83	#9W 37 6.99355E-83		#0# 39 8.91986-83	### 48 6.96371E-83	8794 41 8-15176E-82	8-13644F-02	494 -43 9-120398-92
t description	7	CT.	+				55 •••••••••••••••••••••••••••••••••••		(·	'	' ' C (· ·	· .	٠,	(·	0	r

-8-88832E-84 -1-62691E-84 -8-25948E-85 8-38647E-87	U.E.A.R. M.E. 0.432950E-44 0.43576E-65 -0.487592E-65	0 0 0	0.32178E-84 -0.32178E-84 -0.33188E-07 -0.59512E-06	0.30449E-03 0.837499E-03 0.33665E-86 -0.33665E-86	-6.4299/E-84 0.23311E-84 9.9596E-06	8.21005E-01 8.21005E-01 8.6459E-05 8.28506E-05 8.28506E-05
22.2		- 6 - 864 - 6 - 58 1 - 6 - 58 1 - 6 - 58 1 - 6 - 44 8	-8.652846-87 -8.158326-85 -9.484326-86 -9.4843596-87 -0.481896-87 -0.481896-87	-8.42977E-84 8.23451E-84 8.23451E-84 8.23451E-84 -8.3261E-86 -8.32655E-84 8.32655E-84	-8.15338E-8/ 6.22675E-84 6.22675E-84 6.24359E-8/ -8.24359E-8/ -8.2435E-8/	-0.4496E-94 6.5996E-94 6.26926E-95 6.15941E-96 6.99168E-96 6.99169E-96
				-0.42411E-07 0.95937E-04 0.24034E-04 0.44529E-06 0.44342E-06 0.44342E-06 0.44342E-06	8.15.294E-84 8.25.36E-86 8.27.36E-84 8.47.786E-95 8.43.754E-97	- 15 - 15 - 15 - 15 - 15 - 15 - 15 - 15
-8.34131E-35 -8.34131E-35 -8.34131E-35 -8.3756-67 -8.6756-34 -9.26978E-34 -2.2491E-37	5 n.61961E-N9 7 n.25661E-N6 8 0.26389E-N6 4 0.35839E-N6 4 0.48489E-N6 5 -8.76272E-N5 7 -8.26699E-N6 8 0.88489E-N6	-6.296996-87 -6.382296-87 -9.133926-87 -6.143366-64 -6.26386-85 -6.26386-85 -6.26386-85	0.76182E-8/ -0.26224E-08 b.31334E-04 -8.26375E-06 -0.47249E-06	-6.76113E-87 0.75500E-88 6.98929E-84 8.02874E-85 -8.8774E-87	-8.12829E-8. -8.12829E-8. -8.48568E-84. 0.21397E-84 8.8693E-86	-8. A5846E-88 6.65498E-87 6.65498E-87 6.65456E-65 6.69357E-65
-8.5556E-45 -8.57375E-75 -8.46344E-17 8.38715E-87 -8.58177E-34 -0.58257E-85	5 - 8 - 3 6 2 5 9 E - 10 4 - 10 - 2 3 2 5 5 9 E - 10 5 7 5 9 E - 10 5 7 5 9 E - 10 5 7 5 9 5 9 5 E - 10 5 7 5 9 9 9 5 E - 10 5 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	0.786A5E-85 -0.24641E-05 -0.24641E-05 -0.44564E-84 -0.2524E-85 -0.55284E-85	8.228966-63 6.69/466-86 -8.28168-86 -8.251296-83 -143856-85 -8.268956-85	0.2251E-04 0.14111E-06 0.14111E-06 0.1645E-04 0.1645E-04 0.1642E-06	-0.13991E-84 8.16489E-84 8.16489E-95 8.17124E-07 8.28259E-84 8.1426E-95	8.10679E-64 8.21262E-05 8.21262E-05 8.73869E-06 8.46634E-05 8.25318E-05

the three the the text of the second second

Applications of species with the second

· Car REPRODU 0.21/04E-05 -0.40922E-07 0.20807E-05 0.66397E-07 8.258486-87 -8.276766-89 8.413846-87 8-21/87E-84 8-44/59E-85 8-28828E-85 8.1728/E-84 8.27842E-85 8.16593E-85 6-11/945-05 8-24549E-05 8-16595E-05 0.39641E-#> 0.28615E-65 0.22851E-66 0.28542E-96 0.15421E-84 0.35363E-85 0.23445E-85 -6-10164E-06 9-22189E-85 9-29566E-45 6-18281E-84 8-25974E-85 8-1281/E-85 0-61253E-65 -6-20967E-07 0-15876E-55 -0.18420E-05 1-130518-05 T . 0.21297E-86 8.24325E-84 0.14652E-05 0.20596E-07 -0.1116/E-65 0.12054E-0/ 0.7325E-0 0.17523E-04 0.73749E-06 0.15272E-90 # 17485/E = 84 8 - 1485/E = 84 8 - 13679E = 85 -8-67128E-88 -0-13171E-06 8-20007E-64 8-10-07E-05 -8.42662E-84 -0.32326E-04 8.15946E-04 4 9-11210E-0> 0.14986E-84 0.76817E-06 -8-13953E-84 8-18628E-84 8-97136E-86 9-26/58E-64 9-1894/E-94 9-226/6E-96 0-14296E-09 0-1302/E-04. 0-19741E-09 -8-34/88C-8-7-44648E-6/ 9-50235E-96 0.20514E-04 0.30636E-06 -0.20034E-06 U.10352E-06 W.80026E-85 -0.09539E-87 9-17828E-88 0.11891E-83 6.86259E-85 8.57788E-89 8-19741E-84 8-18876E-84 8-5846E-87 -8-55185E-87 8.76885E-84 0.49185E-84 0.15893E-84 8.25994E-67 8.98765E-84 0.16647E-84 8.20519E-86 0.29272E-85 0.31631E-84 0.12563E-86 -6.53205E-87 -0.39583E-05 0.11789E-04 -0.76549E-09 # - 60195E-67 -0.92522E-07 0.44468E-85 -0.21512E-07 0.34576E-84 6.12669E-84 8-91936E-87 8.50974E-04 #.21674E-03 6.28449E-05 -0.46238E-07 0.5221E-09 0.27082E-04 0.28014E-06 8.24837E-87 8.49982E-84 8.33547E-86 8.34129E-87 0.2469E-83 8.35924E-83 -8.28561E-86 -8.21059E-66 -8.76681E-85 -9.76681E-85 -9.46683E-87 8.54334E-03 8.51217E-06 -0.51274E-07 -8.3753/E-89 -0.12656E-05 -0.12176E-05 -0.75416E-06 0-13888E-83 -6-76288E-06 -0-75158E-88 9-48458E-84 8-23258E-86 -0.2697/E-85 0.12009E-06 -9-89659E-65 9.11254E-67 -# .96896E-#7 0.47687E-06 0.25631F-04 -6.10268E-06 -6.10310F-87 -8-15933F-05 -0-20279F-05 0.38223E-08 -0-1400E-84 -0-1400E-85 -0-1794E-85 -8.36462E-06 -6.79128E-06 -8.5381/E-84 -8.5381/E-86 9-18376E-84 9-99372E-88 -8-62283E-R6 -0.43319F-66 8.42895E-85 -8.14699E-86 -8.482A7E-84 8.25163E-84 -8.48885E-86 -0.52427E-04 0.53773E-06 -9.53337E-06 -6.53738E-96 -0-19771E-0/ -0-12842E-04 -0.22635E-05 -0:54568E-04 -0.29643E-06 -4-14815E-05 ; 0.35848E-04 0.1827E-05 -0.62574E-05 -8.27572E-06 0.86431E-66 -8.53754E-85 -0.31457E-46 -0.26102E-04 11.95/26E-07 8.14645E-95 -8.26237E-05 -8.26559E-66 0.14799E-05 0.14799E-06 -0.14077E-05 0.85358E-05 0.70188F-88 -8.19203E-86 -U.34152E-66 8.44837E-85 -9.57241E-85 -9.15686E-84 -0.24366E-64 -8.25443E-98 -8-15817E-87 8.31692E-84 -8./9668E-86 -8.37178E-05 -8.15345E-84 -6.39302E-05 -0.51914E-05 . 8.18449E.Q4 8-11864E-86 6.86/84E-87 -8.25581E-85 8.35997E-87 1-67114E-115 -0-51714E-04 -4-BR302E-04 -8-226.55E-05 -1-99208E-117 THE STATE OF THE PROPERTY OF T -8-11/61E-07 -8-48/836-85 -8-4996E-85 -0-43.649E-04 -8-38578F-84 -8-38578F-84 -8-28602E-95 -8.49617E-84 -8.45138E-85 -E-13334E-67 8.63215E-85 8-98981E-85 -8.464A9E-A7 -0.21070E-05 -# -75270E-84 -8-46841E-84 -8-17485E-05 8.13984E-87 -8.28372E-84 -4.16882E-07 -0.77858E-04 -0.27818E-04 -9-38763E-05 9.24690E-03 -0.67958E-44 -0.93239E-04 H - 23446E - H 3 - H - 17479E - H 4 H - 29687F - H 6 H - 77436E - H 7 -8.44988F-84 -8.29573E-86 0.16672F-87 0.2483E-84 -0.54863E-84 -0.61135E-86 -0.5617E-88 P-1746/F-37 -11-24173F-n4 8-18464F-86 -8.13361E-A7 -B-48938F-84 -8-48768E-86 8・52554F-84 8 - 13959F-16 8-5229F-84 8.62763F-n4 -8-42558E-05 -0.86536E-05 8.16243F-48 8.19255E-46 9.58467F-84 -0-14272E-64 1.354246-46 -8.17026E-44 -8.39621E-06 1.55412E-14 -8.26882E-84 -8-36618E-06 9-26/59E-40 0.69630E-07 -0.30296E-85 0.20129E-06 -0.52224E-06 8-17797E-86 0.48956F-03 -0.61482E-05 -0.23426E-07 -14126F-82 -0.26592E-85 0.15558E-96 0.14115E-02 8.57367E-#8 -0.834946-65 -8.58588E-06 1.16943E-R6 0.13553E-86 8-10078E-05 8-141R6F-R2 # - 73228F - 86 # 13956F-82 0.30001E-06 9.77243F-86 -0.41446-05 -0.33611F-05 9.92450F-67 -0.564185-85 8.38889E-88 0.68878F-06 \$.13258E-02 9.71658E-05 -0.36455E-05 9.53792E-EA -1.464506-06 9.12070E-12 -1.48452E-15 8.50124E-96 8-11284E-06 4-20778-10 ** 30 K XOX 202 ¢ 57 C ζ

(

0

Q

2				*									
20 - 30 - 30 - 30 - 30 - 30 - 30 - 30 -	6-32651E-89	6-11239E-84	- 6 - 28940E - 65	9.578386.85 9.44897E-87	-8.19196E-8/	8.16998E-84 9.38792E-84	8.13.969E-04	-9.26>66-85 9.25248E-85	8.62885E-05	-8.43514E-96	8.17886E-84	0.12262E-04	-0.19756E-85
0.37825E-86 0.51975E-86 8.44335E-86	-8-57031E-94 0-15075E-84	-0.13115E-64 E-11596E-04	**************************************	6.28624E-05 8.15265E-06	. 0 - \$22398E-06 -0 - \$4318E-55	-6-37559E-84	-0.12289E-04 0.11262E-04		0.19488E-05	8.10733E-05	-8-42823E-84	-9-15268E-04	-0.25084E-04
00.00000000000000000000000000000000000	8.84799E-04 0.41480E-65 8.22641E-07	0.1460/E-04	0.12889E-84	8.55463E-84 E-12666E-84	-6.13865E-05 - 0.05409E-05	0.839896-64 8.781496-65	8.15126E-04	0.11928E-84	8.56836E-64	-0.12457E-05 0.59915E-05	9.76134E-84	8.59456E-85	8.59915E=86
-9-78/795-39 8-343465-36 -8-278485-81	6.45621E-65 6.28594E-65 -6.1365E-86	9.17461E-85 -9.76254E-85 -8.93618E-87	8 - 18422E - 83	0.17982E-03 -0.2342/E-65	8.21834E-84 -8.24412E-86	8.25966E-85 6.35184E-66	8.27842E-05. -0.10773E-86	6.16/12E-65	0.18158E-03 -8.56742E-06	8.24587E-84 . 8.13276E-85	-6.37849E-65	0.16633E-03	8.17534E-63
8-23/78E-84 -8-56817E-86 6-27418E-88	-0.27850E-04 -0.27850E-05 -0.35274E-05	8.75/39E-85 8.35664E-86 -8.23148E-85	-8.50008E-84 -8.54/34E-95 -9.13059E-35	## 58618E-94	8.26253E-04 -0.16982E-06	-6.23739E-65	8.85872E-85 9.24858E-86	-8.55836E-84 -8.13336E-86	8-18855E-84	9.26656E-84 -9.57764E-85	. 6.11614E-04 . 6.52279E-85	8.53918E-85	.8.62884E-84
-8.38279E=R4 8.67192E=R7 -8.13071E=R8	8.28454E-84 8.41977E-85 -8.67367F-85	8.272785-44 8.124415-96 -8.41978E-85	-8.15439E-84 -8.51616E-89 -8.44774F-69	8.16869E-84 8.29432E-86 -8.49493E-85	-0.283838-04 -0./1521E-07	8.28768E-84	0.27173E-64 0.14311E-05	-8.17862E-84	8.15659E-64 8.41598E-87	-8-28627E-84	8.28529E-84	8.27848E-84	-8.23582E-45
# - 13278E - 94 - 4 - 18134E - 85 - 8 - 13846F - 85	-8.5176585-84 -8.517658-94 -8.258488-85	-8-17872E-85 -11-19887E-85 -8-37288E-85	-8-53576E-84 -8-37488E-84 -8-36496E-95	1.62519E-85 -1.6568EE-84 -4.44614E-85	8.12881E-04 -0.15610E-06 -6.16511E-95	-6.68278E-64 -6,48333E-84	-8.28912E-86 -8.36894E-84	-8-63339E-84 -8-33369E-84	4.89515E-85 -8.31847E-84	4.15083E-84 -	-4-66898E-84 -8-56517E-84	4.38663E-86	-6.62728E-84 -
	6.19910F-6.5 -8.18286F-84 f.41288F-96	0.54817E-84 0.74498E-87	0.47261E-44 -8.26825E-84 -8.72746E-86	0.19456E-85 -8-46754E-84 -8-38149E-86	-0.57480E-05 -0.1938E-44	6.18886E-83 -8.79281F-85 8.94997E-87	1.48211E-04 -8.22764E-64	0.414296-44 -0.23575E-84	6.18932E-03 -8.51868F-04	-8.56176E-85	8.1965E-43 -6.1965E-44	6.51889E-84	8.4447BF-84 -8.5367BE-84
2.28135E-85 8.72889E-87	#6W 2: #.37538E-#3 -#.4274!E-#5 -#.21,***E-#6	80% 22 8.11344E-02 8.62863E-85 -8.55175E-86	23 1.11323E-#2 1.28996E-#5	8.11494E-82 -0.53732E-85 6.14692E-86	25 1.35349E-83 1.15778E-85 1.21581E-04	6800 26 0.37165E-03 -0.63159E-05 0.65256E-06	R6W 27 6.31284E-02 8.76936E-05 -0.31079E-06	888 28 0.11223E-02 -8.77665E-85	#09 20 0.11377E-02 -0.43730E-05	38 8.34313E-03 8.12783E-06	31 8-37543E-83 8-65537E-85	8-112336-62 8-338586-65	POM 33 P.11262E-P2 -8.12772E-84

. . .

الا الكار المحاسم والمداري والمحافظ المارات المائية والمكارد والمائد والمائد المائد المائد والمائد وال

territy and the territories and the theory

		-8.64325E-84 8.32645E-84	-8.13342E-64 0.12565E-04	-0.16692E-04	¥.				7	job		jo.							,				•
1 1	1	1 1	ĺ	1 1	6E-04										AD!	C	BL.	Š			, , ,	, and a second s	•
	7/2001	9-12836E-63	6.29524E-84	0.23345E-04	6.43666E-04				,				`			,			,		,		٠
	10	8.54121E-84	8.2165/E-83	8.22872E-83	.8.23341E-63	0.428616-04	;												3				•
		-0.178115-04	0.12725E-85	-8.28376E-A4	-0.3585.E-00	8.541A3E-84	-8.565485-85					-											•
		8-24/82E-84 -(#-2686#E-84	.29481E-65 -(9-17752E-84 -1	-8.34365E-94	9-1A584E-A4	8-11512E-64				·		1 A PR 1 A C 4 C 4 C 4 C 4 C 4 C 4 C 4 C 4 C 4 C						•			•
-0.27971E-04	- 1	-8.63598E-84 B	42868E-05 &	.66278E-84 -8	-13571E-85 P	9-13818E-84 -8	34844E-84 8.	8.26318E-64 B	-8.24293E-84														
	•	1 1	=	1	c		•		- 1	9						·	,						
-8.48856E-04	-#-26318E-84	8.282665-43	8.63533E-44	8.58684E-64	R.21682E-63	-0.56202E-05	8.59148E-64	-6.62155E-04	-8.778286-84	0.51215E-04					***					,			:
		8.37798E-83	8+11658F-R2	# 38 #.116115-#2	1975E-82	0. JARRE-83	e.86339E-84	42 49326E-03	0.53842E-03	1 44 0.47355E-03	0.12485E-03								,				

HERE ARE THE EIGHVALUES A	AND EIGENVEGTONS	97		
# # # # # # # # # # # # # # # # # # #		1.8888479E-81.		
1.000000000000000000000000000000000000	3.3347199E-03.	3.197/424E-81		1
の、他のないないに、これでは、「ないないでは、「ないないない」というののなができます。 と、他のないないは、「ないないない」というのでは、「ないないないない」というのでは、「ないないないない」というのでは、「ないないないない」というのでは、「ないないないない」というのでは、「ないないないない」というのでは、「ないないないないないないないない。」というのでは、「ないないないないないないないないないないないない。」というのでは、「ないないないないないないないないないないないないないないないないない		5.3241888E-81 8.6379285E-81		1
2E-01		9.8326887E-81	30	1 1
. Subst			eĜ.	
CORRESPONDENCE TO 4.CA679386 AS	18160865	9.35813345+83		
/ 10 10 10 10 10 10 10 10 10 10 10 10 10	0593VE-01	.6081884E		 -
-2-4/28/825-81 -4-43238295-81 3-28/23/31E-81 -4-1484/241-81 0-/345674E-81 3-8968466E-81 -4	98577E-82	3	Ó	-
	25489E-81	9.4158386E-01	3	1
	******	20.6.16.17.2.12-6.1		ł
2 E E E			wa	{
-9.95.386.5E-82	.8648855E-02	4.9031451E-01	Z	1
-3.4911/48E-81 -3.43183/5E-#1 -7.9975693E-61	5873852E-01	6.6552919E-81		-
-9.54884675-41 -5.17371315-41 -8.20598215-81 -5.90644175-41 -6.93684365-81 -6.3457942 <u>5-81</u>	-7.4191869E-01	-6.7374431E-61. -9.3329692E-91	₹.E.	ĺ
-4.0529455E-01 -3.6518532E-01 -3.132/445E-01 - 4.44.022E-03 5.3130134E-03 0.01478E-03	338757E-81 .	.2.6891975K-81 4.6549727K-81	PA	
5.045372757575757575757575757575757575757575	48646876-81	9.6285614E-01	Ož	l [*]
			U	ſ
THE STATE OF	844284E-81	4.501.48346-81	II	
5.6288112E-01		2.7715041E-01	Z	
4.5018474F-81 7.7424708E-81 -5.8752942E-81 4 8234448F-81 -7.834448F-81 -7.4437648F-81	-J. #683886E-#1	2.8691754E-02 1.8895366E-01		1
-6.7553564E-B2 -9.8757267E-R2 -7.9457745E-82		4.2292678E-82		1
1.185601AF-81 -1.5868755E-82 -7.28462797E-81 -5. -1.5957.28F-82 -3.4386595E-81 -5.9486389E-81 1. -4.8634785E-81 -7.6681481E-81		5.5653499E-81		1
A514148F 80				
3.1571091E-81 2.404165-81 1.97.82443E-81	1.34639535-81	9.2448528E-01	eterphismus de l'universe de l	{
な。ロインのインのでは、「ロールン・スクラインでは、「・ロインとのグローローな。「ファロストコアース」 「・ロンスルンのドーローな。「ファロストース」 「・ロンスルンのドーロー	3105136E=#1	7.6716151E-62		
-8.5884123F-87 -4.78363BF-81 -3.628499R-81 -7.258973F-81 -6.258977F-81 -6.258977F-81	5672700E-01	5.9208911E-81		1
-4.7686178F-81 -3.4172988F-81 -3.185547F-81 -2.26578F-81 -5.186578F-81 -2.6671677F-81 -2.56578F-81 -2.5657878F-81 -2.5657878F-81 -2.5657878F-81 -2.5657878F-81 -2.5657878F-81 -2.5657878F-81 -2.56578787	3278939E-81	5.3459271E-82 8.2758456E-81		
7				
2. MONE SPONDING TO 1.6728392F 19	3101.	4.k \$6260 \$	Annual An	[7]
	1			

			No. Response	
### ### ##############################	EIGENVECTOR NUMBER 7 CORRESPONDING 10 1-7581530E 14 -1.9800879F-81 0.4284345E-82 -1.1374335F-82 -1.137433F-82 0.4331737E-02 2.8841379F-81 0.4284345E-82 -1.9800879F-84 -4.2208745E-82 -1.1374358E-81 2.77789E-81 -1.1319853E-81 -2.1433781E-81 7.4782497E-87 5.775546879E-81 -4.7469778E-82 -3.47599428E-81 -2.1433781E-81 7.47782497E-81 5.7774748155E-81 -1.986789E-81 3.785559E-81 -2.1433781E-81 3.8458148E-81 7.77777777777777777777777777777777777	6.17637E 10 4.256699E=01 -3. 4.6143628E=01 -7. 2.6595636F=01 4. 5.479934F=01 4. 5.4779837E=01 -3. 6.477986=02 -0.	EIGENVECTOR RUPHER CORRESPONDED 1. 1.156me2.4F is CORRESPONDED 10.3947799E-01 CORRESPONDED	

			atar ingin			
MERF ARE THE MATURAL TREGUENCY MUNDER	: 300 : 300					**
THE TANKE TO THE TANKE THE	THE MATURAL					and the second s
		TYPE MAJURAL FREG THE MAJURAL FREG THE MAJURAL FREG THE MAJURAL FREG THE MAJURAL FREG THE MAJURAL FREG THE MAJURAL FREG				
				62	1	

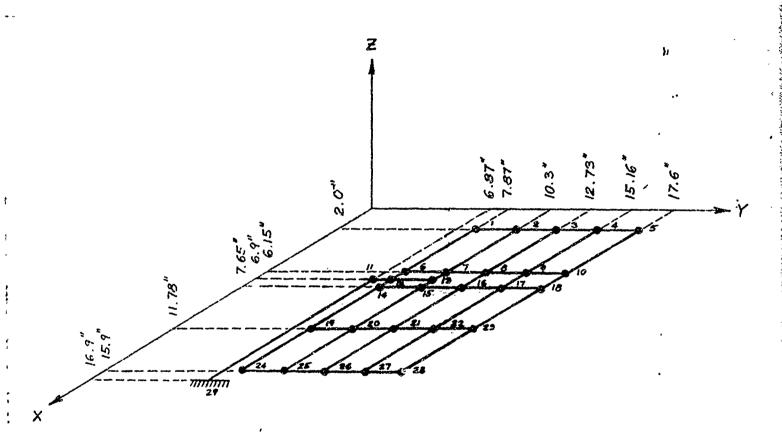
O C

Q

Sample Problem No. 3

Missile Control Surface Hodel (Modeled with beam elements and lumped weights)

Find first five natural modes and frequencies.



Note: Joint 11 is restrained from rotating about y

Lumped Masses

Joint No.	Mass 15.
Joint No. 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29	

Beam Blement Properties

Member	Moment-of-Inertia Area	Torsional Constant			
i - i	inc				
	IIICII				
1-2	0.0009	0.0055			
2-3	0.0009	0.0055			
3-4	0.0009	0.0055			
4-5	0.0018	0.0055			
6-7	0.0164	0.0300			
7-8	0.0164	0.0300			
8-9	0,0164	0.0300			
9-12	0.0164	0.0300			
12-13	0.0160	0.0300			
14-15	0.0147	0.0280			
15-16	0.0147	0.0280			
16-17	0.0147	0.0280			
17-18	0.0147	0.0280			
19-20	0.0053	0.0280			
20-21	0.0053	0.0010			
21-22	0,0053	0.0010			
22-23	0.0053	0.0010			
24-25	0.0031	0.0006			
25-26	. 0.0031	0.0006			
26-27	0.0031	0.0006			
27-28	0,0031	0.0006			
1-6	0.0013	0.0026			
2-7	0.0027	- 0.0054			
3-8	0.0027	0.0054			
4-9	0.0027	0.0054			
5-10	0.0026	0.0029			
6-12	0.0503	0.1000			
12-14	0.0503	0.1000			
7-13	0.0255	0.0510			
13-15	0.0255	0.0510			
8-16	0.0380	0.0750			
9-17	0,0380	0.0750			
10-18	0.0377	0.0750			
14-19	0.0017	0.0034			
15-20	0.0035	0.0070			
16-21	0.0035	0.0070 ·			
17-22	0.0035	0.0070			
18-23	0.0017	0.0029			
11-12	100.0000	0.0100			
11-29	0.3200	0.0790			
19-24	0.0017	0.0030			
20-25	0.0017	0.0030			
21-26	0.0035	0.0070			
22-27	0.0035	0.0070			
23-28	0.0017	0.0029			
•					

$$E = 3 \times 10^7 \text{ psi}$$

 $V = 0.3$

Listing of Input Data Cards

```
MISSILE CONTROL SURFACE HODEL WITH 29 JOINTS
 AUGUST- 1968
   29
          2,
              45
                                       24
                      Ü
                                   1
    1
30.
            0.5
                        ñ.
                                    0.
            2.0
                        7.A/
    •3
            2.0
                        10.3
     ţ
            2.0
                        12.73
            2.11
                        15.16
    6
           2.0
                        17.0
    6
           6.15
                        7.81
    7
           6.15
                        10.1
    я
            8.15
                        12./3
    Ç
            6.15
                        19.16
   10
           6.15
                        11.0
   11
           6.9
                        6.R/
                        7.81
   12
           6.9
           6.9
   1.3
                       111.5
                        7.8/
   3 4
           7.65
            7.65
                        10.5
   15
   16
            7.65
                        12./3
            7.65
   1/
                        15.16
   18
            7.65
                        17.5
   j ų
            11./8
                        7.8/
   20
            11./8
                        10.0
   11
            11.78
                        12./3
    17
            11.78
                        12.16
    13
            11.78
                        1/.5
   14
            15.9
                        7.8/
   25
            15.9
                        18.1
   26
            15.9
                        12.73
   17
                        15.16
            19.4
                        11.0
            15.9
   'nЯ
   29
            16.9
                        6.81
   13
              0
                       1
    10
                1
                       1
    Ý
            11.115
    2
            0.11
     .$
            0.115
     4
            0.125
    6
            8.196
     ۶.
            0.155
     7
            0.305
     R
            0.305
     Q
            0.305
    1 13
            8.165
   11
            0.06
   12
            0.165
   13
            0.005
   1 4
            0.183
   15
            0.325
            0.31
   1.6
```

0.28

```
0.14
   18
   ęq
            0.062
            0.078
   211
   11
            0.0/8
   1,2
            0,078
   43
            0.08
   24
            0.033
            0.051
   15
   16
            0.051
   13
            0.051
   cA
            0.042
   "Ÿ
            0.050
                        0.0855
                                          1
            0.0009
A.
                        4.4455
                                          1
                                                2
n.
            0.0009
            0.8009
                        11.0055
n.
            0.8018
                        11.11155
ø.
            0.0164
                        4.23
0.
n.
                        # . 43
            0.0164
                        0.13
a.
            U.0164
                                                     10
                                                9
            0.0164
                        0.03
Ц.
                                                     13
                                          Ĺ
                                               12
            0.016
                        0.43
                                          1
                                               14
                                                     15
            0.014/
                        0.028
                                               15
                                                     16
                        9.928
            8.0147
                                          1
                                               16
                                                     17
            0.0147
                        0.028
                                                     18
                                          1
                                               1/
                         1.128
            0.014/
                                                     211
            0.0053
                        0.051
                                               14
                                               211
                                                     21
            0.0053
                        0.001
a.
                                                     22
n.
                         0.091
                                               21
            0.0053
                                                     23
            0.0053
                        0.091
                                               22
A.
                                                     25
            0.0031
                         0.006
                                               24
A.
                                               25
                                                     26
            0.0031
                         0.006
                                                     27
                                               50
            0.0031
                         0.006
                                               2/
                                                     28
                         0.006
            0.0031
                                                       6
                                          1
                         0.0026
                                                1
            0.0013
                                                       7
                                          1
            0.002/
                         0.0054
                                                2
n.
A.
                         0.0054
                                          1
                                                       8
            0.0027
                                                       9
            0.0027
                         0.6054
0.
                                                     10
                         0.0029
            0.0020
a.
                                                     12
n.
            0.0503
                         4.1
                                          1
                                               12
                                                     14
                                          1
            0.8503
                         0.1
                                                     13
                                          1
            0.0255
                         0.051
                                                1
                                          1
                                               1.5
                                                     15
            0.8255
                         P.851
                                                IJ
                                                     16
             0.038
                         0.075
                                          1.
                                                y
                                                     17
            0.038
                         0.075
                                               10
                                                     18
             0.0577
                         11.075
                                               14
                                                      19
             0.0017
                         0.0034
n,
             0.0035
                                                      20
                         0.007
                                               15
n.
n.
                                                      21
             0.0635
                         0.007
                                          1
                                               10
                                                      22
n.
             0.0035
                         0.007
                                               1/
                                                      23
(I .
                                          1
                                               18
             0.001/
                         0.0029
                                               11
                                                      12
             100.
                         0.01
11.
                                                      29
                                               11
                         0.079
             0.32
```

0.	0.001/	0.003	1	10	24
ß.	0 044		_	47	2 7
	0.0417	0.043	1	20	25
n _	0.0035	0 0 59	_		6.5
	0.0000	0.097	1	21	26
0.	0.0035	4	4.	4.1	20
'' 	ちゃりじょう	0.007	ſ	22	27
n.	8 444		•	<i>e</i> 2	61
** •	0.001/	0.0029	1	2.4	25

Program Output

The state of the s

# 0 P E R 1 1 E S = 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	MMOBE & 9 DEFY = 1 MLUMP = 29
N.A.T.E.R. I.A. I. P. N. O. P. E. R. I. I. E. S. GOOGGGGGGGGGGGGGGGGGGGGGGGGGGGGGGGG	OBE 2 9 MKEV m 1
N T E T A P R P E R T E S S S S S S S S S	espectores and the second seco
D N C O O R N R R E S	
1	
7 7.48888 12.74888 2 2.48888 2 2.48888 2 2.48888 17.54888 2 2.488888 2 2.488888 2 2.48888 2 2.488888 2 2.488888 2 2.48888 2 2.4888888 2 2.488888 2 2.488888 2 2.488888 2 2.488888 2 2.488888 2 2.488888 2 2.488888 2 2.488888 2 2.4888888 2 2.4888888 2 2.4888888 2 2.4888888 2 2.48888888 2 2.48888888 2 2.4888888 2 2.48888888888	
2. Made 15. 17. 5 M 8 M 8 M 8 M 8 M 8 M 8 M 8 M 8 M 8 M	A
A h.15000 7.0.0000 7	
1	
19 6.15488 17.64888 17.64888 17.64888 17.64888 18.38888 18.38888 18.38888 18.38888 17.648888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.648888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.648888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.648888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.648888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.648888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.648888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.648888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.648888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.648888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.648888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888 17.64888	
13 0.90000 7.4/000 13 0.90000 7.4/000 13 0.90000 7.4/000 14 7.6/0000 7.4/000 15 7.6/0000 7.2/0000 16 7.6/0000 7.4/0000 17 7.6/0000 7.4/0000 23 11.76000 7.4/0000 23 11.76000 7.4/0000 24 15.90000 7.4/0000 25 15.90000 7.4/0000 26 15.90000 7.4/0000 27 15.90000 7.4/0000 28 15.90000 7.4/0000 29 18 7 8 5 7 8 4 1 8 7 C 0 0 5 00000000000000000000000000000	
13 6.9888 18.38888 14 7.65888 7.87888 15 7.65888 15.75888 16 7.65888 15.75888 20 11.78888 15.75888 21 11.78888 15.75888 22 11.78888 15.75888 23 11.78888 15.75888 24 15.98888 17.64888 25 15.98888 17.64888 26 15.98888 17.64888 27 15.98888 17.64888 28 16.98888 17.64888 29 18 7 8 E S 7 R A 1 N 7 C G D E seessees	
15. 7.65998 18.59988 16. 7.65998 12.73689 17. 7.65998 12.73689 29 11.76998 12.73699 29 11.76998 12.73699 29 11.76998 12.73699 29 11.76998 12.73699 20 11.76998 12.73699 20 11.76998 12.73699 20 11.76998 12.73699 20 11.769898 12.73699 20 11.769898 12.73699 20 11.769898 17.66988 20 11.769898 17.66988 20 11.769898 17.66988 20 11.769898 17.66988 20 11.769898 17.66988 20 11.769898 17.66988 20 11.769898 17.66988 20 11.769898 17.66988 20 11.769898 17.66988 20 11.769898 17.66988 20 11.769898 17.66988 20 11.769898 17.66988	
10 1.05000 15.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.15000 10.150000 10.15000	\$P
1.45000 7.65000 1.45	
11.78689 10.58888 11.78688 12.75888 12.75888 12.75888 17.58888 17.58888 17.98888 17.588888 17.58888 17.58888 17.588888 17.588888 17.588888 17.58888 17.58888 17.58888 17.588888 17.58888 17	
### ### ##############################	
17.90884 19.37889 17.90884 17.7389 15.90884 17.6488 15.90888 17.64888 16.90888 6.87888. 84.00888 6.87888.	
# 1	•
15.9888	
H T R E S I R A 1 N I C C C E *****************************	
40. C Wishland Adul X	
•	**************************************
NUMBERS FOR	7X:0 20:X7
ACCIAL THE COURT NO.	
5	

=

.

,

	4	6		17	18 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1 / 20	2.2	. 23	25	27	28	1 8 # 1 5	1100						RO.	707											
15		0 =	 (page 4	 17	£1	2.0	/1 22	23	25	22	28	 -		8.1158	# * * * * * * * * * * * * * * * * * * *	8.3550	1000000000000000000000000000000000000	6.0878	6.1654 6.1654 6.1654	9.610	9007.	9.8424	*****	9828	カラウェーカラウェーカ	9.0318	B. 6428	2000	REK! PROPE	€ .	

.

	-	-			•			-													İ				1	l					1				
The second secon	-												TO	2		30	R.												-0.10.3825 b3 -0.23/455 0.1 -0.139175 00		.29345E	-3.58225E-01		8.41.36E 88	
Company of the second second	-							Z	ar			- 1		1	ŀ	-			Ţ		4								4 0.18394E 94 3 -8.12255E 95 8 -9.47108E 08		0.13471E	-0.38642E	8.875.8cE	3 8.13187E 81 1 -9.68894E 88	
C. CARS CARS TOP TO A STATE OF THE STATE OF	·	•									K	3	T.	3	C			حد [*] ور				•					*	·	85 6.49571E 9 85 -0.25693E 6 08 8.31698E 6		#5 #.28599E	7	5	83 -8.39956E	
1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	, ,	.	6 #	15	36	1.0	2 2	22	25	12	38	7	n o	, 37	1.6	15	16	10	19	21	22	12	62	25	27	2.0	* * * * * * * * * * * * * * * * * * *		6E N3 -0.19469E WE 04 8.64852E 7E 08 -0.53375E		7E 84 -8.36624E	90	2	1E 00 -8-49387E	
-	ę	~ «	•	=	41	7.	° 80 ™ N	22	12.6	24	27-	2	. •	æ 4	12	13	<	18	4 4	22	7 -	3.1	11	20	22	23	# L. L.		0.18516E -0.30348E 8.37647E		0.41947E	'I *	6-51561	0.11340E -0.30371E	
8.0825				6.4788	8.8786 1 8.8788 1	# - #24.5	N. C. B. A.		4. C. S.		# . # # # # # # # # # # # # # # # # # #				989	# . 6 5 1 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	8.8750	- [# 4962.3	8:98/0 1	8.0878 1 4.8620 1		6.8748 M	1 0700.0 1 0700.0	.9878		LARST		-8.21434E 84 -8.54610E 84 -8.24938E 86		-0.14128E n4	€	-	-0.28985E 03 -0.54835E 00	
A. 941.8	3	2.0104	8.8164	8-8147	0.8147 0.8147	4.8147	8.685.8	# . # # # # # # # # # # # # # # # # # #	- C - C - C - C - C - C - C - C - C - C		n. no.51	8.5507	0.9027	7.82.E	9.458.5	A. 8255	# CO C C	4.0372	8.881/ 8.8835	8.86.5	9.4017		n.3288 n.881/	6.0027	A.8835	1.0017	TRIARGU		#.433#3E #4 #.19325E #5 #.3#5#3E #1		A.54.12E 84	11.4354BE N1		n.23399E nJ n.11313E 41	4 G L:
	:		5 0	•	**	•	•	• =	•	•		•		•	•				<u>.</u> .	•				• 5		1.	1) F		-0-16368E 64 -0-61275E 8		-#.21823E n5	6-10761F n2	.23913E	-0-15187E n4 n-64389F n1	10 350 00
*	2	7	Ø \$	16	12	7	.15	17	4.9	20	22	23	25	27	. Bc		32	.33	T 16	36	38	39	4.1	42	44	45	3 C E 8	•	8.24871E 82 8.28821E 81 8.13291E-81	•		0.13398F #2	3 3819F 05	9.12282E 84	40 301031
	a	3	` O	ļ C	}		1	<u> </u>		C	, (,			I	i		72	}= }=	į				-	2					707	1	202

•

		:																						₹		ŀ						JOIRT 2	
																																J. MAJ	,
	 > c	11	1.3 1.4	15	17		19 28	71 22	23	25	27	2	·	100	0.50	1961	1556	1856	650	1400	1050	1830	100		1748	1740	1.00	510	.216. .518	E. +42#			
	3.0		13			18		21	23	25	27		10.	2			71	E (200		Element HO.	

~ - -

÷.

.

440

Ta Charles XX	,	,		1			,		:	ı.	•		•						,			1		•						•		_	_						ţ.
a Local First Methods of a company of the control o					-	AND THE RESERVE OF THE PROPERTY OF THE PROPERT			, , , ,				31	4	X.	P	RO	יס <u>ו</u>		CI	Bī	1	5				•			6.18582E	-6.21/45E #1 -\$.13917E ##	-29345E	-0.32691E 00 -0.50225E-01	•	-#:16158E 84	• 41.386E	-	-0-19458E 94	and the second second
A THE STANDARD WITH WANTED PARTY OF THE STANDARD OF THE STANDA		*							·							3 3 3	.5	Z		ş.										0.18394E 0	-0-17259E 03		-9-11234E 05		2	-1-318/E 61 -0-68894E 66		8-6431GE 03	;
de Commendador de la Commenda de La											S						, , , , , , , , , , , , , , , , , , ,		*										×	8.49571E	**************************************	1.28599E	-0.15365E 03 -0.11971E 81		6.30681E	-8.39956E 83 -8.18747E 8%		0.46815E 94	
•		<u> </u>		14	•	117		21 .	7.2	25	26		•		2 5	23	2.4	15	17	18	1.9	21	22	23	29	24	25 26	27	1 X 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		-8.53375E 88	8.36624E	-0.02565E 63 -0.27459E 01	Į		-8.4938/E 83 8.19166E 82		-8.21412E 05	
* *** *** *** *** ***	į			***************************************								`	- 8													0		6. 17	五 元 8 8	-18576E	# -0 - 58348F #4 # - 57647F 88	.41947E	0.28311E ne		.31501E	-9.30371E 00		8.26682E 04	
	一	Ť		W. W. W.		8.8748 2	20000			E SEAS			1 2784.8	1.0074		0.1400	1. 9291.2 2. 4. 7. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2.	8 - 8 5 1 S	# 1 2 2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4		8.8834 1 6.6856 1			8.8829 1	0.0798 1	1		4.69/6 1		11434E	-8.24936E 88		-0.36751E #D			-8.54835E #0		0.28809E 04	1
2 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	9.491.64	2000		0.8147	0.4247	6.8347	-9.8147		800885.5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0 . EH.S.	8.88.S1		2:001 4 0:002	2.00.0	n - 887/	n. 858.5		n. R.255		0.5377	#:091/	E - 1 0 0 3 5	0.0035	# # # # # 17 # # # # # # # # # # # # # # # # # #	0.3200	71000	# . # 6 % 7 . # . # 6 % 5 .	3.8835 8.8817	9 2		E-Jasade 41		u.43548E #1			#-11515E 41		-4-15172F BS	
	•			•	9.	•						B .	•			•	48.40 6.40	•			•	9.0				• •			2 E		-8.61275E #8	-# . 21823E #5	8-18761F n2			6.84389F 81		# 13598E 45	
ŗ wo	, . (• •	1.	11	12		\$ C		18	19	. .	22	10	25	27	80	90	22	33	*	3.6	37	@ 0	.40	41	63	4.4			0.28821E #1 0.13291E-F1	8.29222E	0.13598F #2		7910E	-0.11290F 83		5-8-4 18 0	
0	-	•)		C			,			 	,	, ς , ς		} '\ 		;		ا، 					72:	<u></u> ;					4 '		 *	1		1	1	ě	:4	

, t, ...

The second control of the control of

the Shake all literal and the 1880 198 to the contract of the

4-1-4													. !		
	-8.32940E 33 "	6.54417E 07	6.29/76E 6/ -#.86918E 82	-8.36673E \$6 8.12486E 83	-0.4369JE 86 8-180/9E 83	-6.25/10E 66 0.15676E 0.3	-0.23V64E 84	-6.4056BE 04	0.2424/E 84	-8.34254E 83	-e.23/27E es	-0.23474E 0.3	**. I 2 4 8 2 E 6 J	-8.14365E 83	-0.87764F B2
	19 3789951 · 8	#.56#BIE #6.	-8.70567E 05	6-18689E 86	0.13469E 86	0-18021E 00 0-60736E 02		8.33892E 95	9.53419E B>	0.15803E 64	#.14351E #4	9.69363E 84	-0.55016E 02	* 8 - 4 355 82 - *	0.25735E 03
-8	-6.15418E -8.12745E	-0.11222E 98.	-0.67228E 07	00.141.48 00.725.40 00.725.40	0.29379E 65 8.25777E 62	0.1207aE 05	8.24283E 85	9.568888 94 8.118936 82	-8.63838E 83	0.13163E D4	-0.36121E 63	3-18181E 84	8.25519E 84	-6.79218E 61	8.39668E 84
	-6.17727E 85 8.44764E 89 8.93892E 81	-V-23869E B6 8-2577/E 81	8.28952E 86 -8.13/44E 84	-0.29808E 65 6.46531E 65	-f./0165E 04	-0.64281E 84 0.53886E 81	-0.98688E 65 8.13978E 85	-8.34675E 85	-8.2997/E 85	-8.35568E 85	-6.58738E 05	-6.39128E 95	-6-39328E 65	-8.28817E 05	-8.72441E 84
0h78ê.	4 8.4455E 84 5 8.9847E 84 8 -8.13536E 81	マエ 山のパグケン・の	2 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	8-15349E 85	8.13956E 85 8.19243E 85	0.1417/E 84 6.98549F 84	6.29202E 86 -8.99718E 02	0.13921E 86 0.21637E 63	4.81836F 84 -8.22398E 83	0.25987F 04 -0.10761E 83	6.25498E 84 -8.88296E 82	8.14139E n4	0.17458E 84	9.22157E 84	0.10196F 84
9	# 17191E # - 10055E # - 1 . 95181E #	-4.21837E N5 0.14826F 84 -9.1717E 62		-6.13/65E 46	**************************************	-6.17238E A4 8.98481E B5	-8.21365E A6 -8.17638E 83	U.20220E 06	-6.28988E 65 -6.78512E 84	-8.17532E 05	-8.14948E 85	H. V1798E R3	8.12432E 64	.0./R258E 83	-8.c7282E 04
7 116 nt	3 -8-46181E 02 7 -8-27465E 83 2 -8-51689E-03	5 1.94104E 19 5 -1.66798E 14	1.31141E n6 1.59182E n4 1.898.44E 62	- 1-76278E 85 	1 1.284A5E 45	##63475E 94	2.379934 96 8.88465E 93	-n.1125.3E n8 e.21816E m3	6.50295E 87	4.84537E 85	H.99A21E R5	#-67745E #5	24607E B3	-6.27658E '83	#-27698E 05
n h1.0 38	0-1498#F 6-11-47498F 6-11-4749	74787F 16 9 4-15897E 05 1 -6-56851E 02	7 -11-50143E no 6 8-51969F 85 4 8-23713F 03	3 8 5		-0.44139E 64 -0.81726E 82	-8.19994F 87	-0.16863F 07	#.52655E n6 -#.13694E #3	-8.68926F 96	# 44449E # # # 95839F #4	-8.43355E A6 8.51971E A4	-8-50137E 46	0.41343E 04	-9-47934F #5
The Partie of	80W 5 0.1716 05 0.30594E 07 -0.56738E 02	#0H 6 67807E BY	#0W 7 0.43531E 07 8.17239E 46 6.13199E 8	20 30 30 30 30 30 30 30 30 30 30 30 30 30	ACT CASTACT OF PROFESSION OF P	40M 18 8.37156E 86 8.15375E 82 -8.28826E 84	2 R.17848E B7	#0# 12 6.25.495E 08 0.27639E 84	#7W 13 0-13641E #0 0-14841E #3	#OW 14 8-62167E #7 8-17107E #3	#ON 15 8.43185E 87 -0.59989E 83	#D# 14 0.04315F 96 -8.28677F #3	#68 17 0.76125E 86 -0.39111E 83	904 14 9.31213E 86 -9.33854E 83	90H 10

0.35523E 05 1 24 0.15548E 05 1 25	-8.47169E 05 8.78831E 02 -0.27452E 05	-8.295025E 02 -8.29181E 83	8.25947E B5 B.13618E B4 B.86768E B5	0.20675E 04 0.31995E 04 0.39857E 64	-8.13399E 65 -8.14788E 85 -8.76779E 84	8.31878E 84 8.39842E 84	8.66975E 6.5	
DN 26 B. 27 B. 27 B. 74114F 95 DN 28 B. 15845E 95 E. D. C. E. P. E. D. C. E. P. E. D. C. E. P. E. D. C. E. P. E. D. C. E. P. B. 15101E-83 B. 32311E-83	-0.27225F -0.27225F 0.38422E -0.18272E	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	.55257E-03 44482E-04 .14501E-03	E X 1 B 1 L 1 - 0 - 36663E-64 - 6 - 12599E-64	9.51564E-04 6.96562E-05 -0.39581E-63	1 X 0.79629E-04 0.32239E-04	0.10505E=63 0.54675E=64 -0.35934E=04	8.12639E-64 0.77662E-84 -8.34152E-84
# 40/735-#3 # 40/735-#3 # 2983/65-#3 # 3 # 3 # 3 # 5/6885-#3 # 6/68885-84	8.556665-43 8.44482E-84 -8.28785E-84 8.92198E-83 8.28742E-83	4.62917E-83 4.43517E-83 8.64638E-84 8.18782E-82 8.44821E-83	0.69/54E-03 0.15162E-03 0.16262E-03 0.16966E-63 -0.39228E-63	8.767725-04 8.988165-04 -0.393845-03 0.264895-03 8.353675-03	8.17215E-03 8.18953E-03 -0.3146E-03 8.43863E-03 -0.9902E-03	8.26864E-83 -8.28964E-83 -8.22994E-83 8.6132E-83 8.68194E-83	0.36271E-03 0.37412E-03 -0.14684E-03 -0.16977E-03 8.21456E-03	9.45575E-03 -9.18582E-03 -9.63413E-84 -8.85273E-05 -8.21599E-04
8.17342E-02 8.31941E-03 8.52875F-03 8.59502E-02 8.80399E-04	0.14961E-02 0.62982E-14 0.7769E-33 0.15278E-03 0.3749E-03	1	8.55738E-n3 8.52283E-n3 -0.16456E-n3 8.78356E-n3 n.18418E-n2 n.22577E-n3	H.61106E-03 H.77332E-03 D.6H875E-04 D.1143BE-02 B.14641E-02 O.53499E-03	8.87635E-63 8.10425E-62 8.28915E-03 8.15147E-62 -0.13716E-03	0.11439E-02 0.5209/E-03 0.5209/E-03 0.91140E-05 0.15715E-03	6.98282E-84 6.65915E-84 8.1205/E-83	0.95134E-04 0.29243E-03 0.41204E-03
24126F-84 8-24126F-84 8-395698F-84 -9-396698F-84	0.40431F-64 B.649815-84 -0.14428E-84	1.247435-84 8.982095-84 8.188445-84	0.10085E-63 8.11562E-63 8.34128E-64	8.12246E-83 -8.12748E-04 8.51584E-04	0.87419E-65 9.12557E-04	0.19162E-04	0.44596E-84 8.63122E-94	0.14287E-04

A CONTRACTOR OF THE PROPERTY O

*											×			-	- -	
4.22".U.L.B4	8.38981E-43 8.15/68E-83	6.82169E-85	0.14546E-02 0.1880E-02	8.87428E-05 9.91169E-05	6.44497E-04	. 0 - 22 / 38 E - 0 .	*.12/20E-t3	0.45548E-03	6.13662E-65	9.48542E-03	P.44213E-14	0.500%JE-0.3	8.9793E-8.5			
.00-Boar (6+4)	8.22176E-85	8.250/8E-83	9.18864E-8.2 8.67529E-8.3	8.91256E-85	0.10170E-04	8.13964E-03	8.18192E-83	\$.35755E-8J	0.78334 <u>E</u> -6J	6.15324E-05	0.50229E-03	8.62916E-63	8.00668E-#3	8-14080E-02		
0.20c.0E-6.	8.64986E-84	8.1183/E-85	8.73584E-63 8.54577E-65	6 - 90214E-89	0.12057E-03	0.44221E-04	3.76669E-84	0.26902E-63	0.61135E-83	#.11366E-62	£.16546E-£.	\$167185E-#3	8.83475E-83	0.122236-62	8.18414E-82	
U.1.048E-25	0.22741E-03	#.98216E-64	5.4866E-85	9.89281E-85	6.9%145F-84 8.09835F-84	8.432285-84 8.419515-84	0.51439E-84	0.17235E-63	6.44825E-63	0.67679F-03	#+15#29E-#2	0.72276E-03	0./A122E-05	. 184816-82	8.15482E-82	6.22972E-82
u475E-u-	0.69813E-04	0.31941E-03	8.1356JE-83 8.13954E-82	0.87436E-#5	9.69815E-64 8.44586E-84	0.33984F-83 6.27759F-95	0.12548E-93	0.79746E-84 6.48289E-83	0.270586-63	0.61841E-0.5	E-11453E-02	8.544KBE-85	2 - 18 5 4 8 F - 18 5	8.84936E-83	0.12361E-82	0.18549E-#2
U. 34232E-113	0.89279E-05 0.10133E-03	0.95140E-04 0.52955E-03	N. 19464E-82	#.87421E-85	8.44489E-84 8.19179E-64	#-22/4nF-#3	8.19696E-46	8.47834E-83	8.81837E-83	8.J6676E-83	8./9665E-83	A.34451E-N3	8.78539E-83	6.65399E-83	8.93893E-83	0.14246E-42
# 486 5 # E - # 3	11.74514E-11.3 11.20718E-114 11.65495E-11.3	n.98211E-n5 n.28544E-n3	# . 1705.7E - n.5 n . 785.48E - n.5	8.88349E-85 8.98222E-85	8.19164E-84 n.1795BF-85		4.74754E-84	8.32567F-83	8.74692E-43	8.17238E-83	4.46107E-A3	4.34828E-83	4-62218E-83	4-1046BE-R2	8.62835E-#3	a.10065E-62
11.1596UE-03	11.57455F-0.3 10.73735F-0.3 10.48791E-0.3	11-11819F-02 0-48296E-04 8-99386E-03	0.699947F-05	# . #7470F-65 # . #8790AE-65	8.44498F=84 R-95151F=84	B.44512F.84 B.41280F-83	0.49438E-44 0.78564E-R4	8.23334F-83	0.57586F-83 0.47615F-83	8-11824F-42 8-91499F-83	8.13944E-83	8.36295F-83	8-54187E-43	8.93946E-93	8.14887E-82	9.60348E-83
8-27-10nt-n3	7 ROW 6 5.35F-03 0.56080F-03	#0W 9 0.3727F-M3 0.1086NE-R2 0.74349E-R3	ROW 10 1077E-02 0-1477E-02 0-65/52E-04	#OW 11 6.87#36F-#5 6.66399F-#5	#UN 12 #-10143F-84 #-69873F-84	#6W 13 8.13573F-83 6.31957F-83	6.19259E-84	#96 15 9-14188-83 8-11118F-85	#0# 16 8.48.73E-#3 8.48.73E-#3	80N 17 8.63778F-43 8.65978F-83	868 14/28E-92 8-11661E-62	80W 19 6.39517F-83 6.57878E-83	80W 28 8.45877E-83	86H 21 6.73315E-03	#84 22 6-350446-92	8-39176E-02

	26 17584E-02								•
	·	A.19734F-82	8.21473E-82						
	27 234965-82	0.270025-03							
	28								
	347481-114								
	REDUCER		IRIANGU		0 # 1	A T R 1		the state of the s	
	30000E-01	-							
			3	2 2 6				* ,	•
1380 10 10 10 10 10 10 10							•	•	
	2						, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
1. 1. 1. 1. 1. 1. 1. 1.	20001		•	•	•	: :		G &	• •
13 15 15 15 15 15 15 15		:	•	. 0	•	•	•	•	
4	3		•						
Color Colo		9.	*				-		P.
	-0.4	•	N.	•	-1-	•	9.5	9	•
	*				•	•			
	AZSRBE NO	• • • •		. .	• •	•	 	•	***
		•	:	•	•	•			
	5 19680E 80	•		3		•		•	
			•		•		***	•	•
					•	••			
	6 15500E 00	9.	•	5		•			
		9.			, , B 3	• •		• •	* (
			•	£.	•			A CONTRACTOR OF THE PROPERTY O	
	:	·	3	,		•			
		• (e 1	0.	•	•	•		9:
		•	. 6	:	9		•		
		0.	•	•			•	÷ (1	
50.0E 0 0 8 0 8 0 8 0 8 0 8 0 0 0 0 0 0 0 0		•	•						
	,	•	ů,		-	•	***************************************	_	
					•	. 0	•	•	•

· •	•	•	•	ě	•	•	÷	å	ğ.				,					
	•	65 89	00		•	0.		·	•	· e	÷							
• •	:	÷	::		90	•	•	•	.	-	•	ô g						
* :	:		•	÷	•	•	•	·	•	•	÷	*	•					
•	:0			• • •	9 0	•	•	•	·	:	:	1.	•	6				
2			2	• • • • • • • • • • • • • • • • • • • •	•	i	•	•	ż	9:	0.	, .,	•	ě				
	= =	. 0	0	£ €	- .	•	:	•	*	4	•	e	·	•				
- 1 de la 1 de	70X 11 8.68038E-81 0. 6.	0-14548E 88 0-	ROW 13 1-50000E-62 0. 0.	90W 14 0.16388E ne e.	ROW 15 6.32566 00 0.	ROW 16 0.31069E 80 8.	60W 17 0.29000E 00 0.	204 10 0.14688E 00 0.	86W - 19 0.42#4£E-63 0.	ROW 20 R.ZERRE-91 C.	8-78898E-81 8.	6,78101E-01 0.	8.88888E-01 6.	9.23998E-01 5.	0.5100E-01 0.	468 24 2.51210E-01 1.	903 27 9.9100 X-01 0.	2- 2- 2- 2- 2- 2- 2- 2- 2- 2- 2- 2- 2- 2

(

C

r#s T EIGENVECTOR NUMBER 9 C. 7.4101581E 0/ CGRRESPONDING 10 7.4101581E 0/ CGRRESPONDING 10 7.4101581E 0/ CGRRESPONDING 10 7.4101581E 0/ CGRRESPONDING 10 7.41015816 0/ CGRRESPONDING 10 7.41015816 0/ CGRRESPONDING 114E 0/ CGRRE -3.1443082F-N2 1.7493449F-81 3.8784856E-N1 2.372244RE-N1 5.3674586E-N2 -1.88891761E-81 -1.8526941E-N1 1.92826AFE-N2 -1.88891761E-81 -1.8526941E-N1 1.92826AFE-N2 -1.6654638F-N2 -1.66546A9F-N2 -1.66546AP-N2 -1.66546AP-N 79 C

D

OT

ľ

•

(

(

(.

(

C

0

	AND THE PROPERTY OF THE PROPER
-	HERE ARE THE MATURAL PREGUENCIES
,	MATURAL SPECIESCY MUNES 1 15 60.065
	MATURAL FREQUENCY MUNRER 2 15 129.890
•	MATURAL FRESURECY WORDER 4 15 474.760
	MATURAL FRENCHEL MUNICE 5 19 2496.858 MATURAL FRENCHENCY WORNERS 5 18 3096.858
	THE HATURAL FREGUENCY NUMBER 8 1238-934 CPS THE HATURAL FREGUENCY NUMBER 9 15 1370-042 CPS
· · · · ·	
0	
(
0	
elevities.	
Manager as a	80
0	
(
,	
,	
(
0	
•	
(
, ` •	
0	
r	
_	

APPENDIX B

Program FLUENC Listing

```
FORTRAN DEGK
           PROGRAM FLUENC-FOR GENERATING STIFFNESS, FLEXIBILITY AND MASS
CHAIN
4
           MATRICES FROM PLANE GRID BEAM AND TRIANG. PLATE ELEMENTS
      DIMENSION TITLE(24), YM(10), PR(10), GE(10), BENS(10), X(50), Y(40),
    11NR1(58),NR2(59),NR3(58),N1(59),N2(50),N3(30),NBC(9),DCS(2),
    1STM(6,6),SMM(5,6),PLTK(9,9),PLTM(9,9),SSTF(11325),SM(11325),
    ERSMASS(150),A(11325),VALU(9),TEMP(50),B(158).C(100),DUM3(150).
    1F(150,3), 10UM4(59), JMASS(50)
      INTEGER OUT
     EQUIVALENCE(SSTF(1),SM(1),A(1)),(STM(1.1).SMM(1,1),PLTK(1,1).
    1PLTM(1,1))
 1046 FORMAT(12A6)
 1001 FURNAT(1615)
 1082 FORMAT(AE10.3)
 1983 FORMAT(10X,2F10.3)
 1004 FORMAT(3F18.3.315)
 1005 FORMAT(E10.3.015)
 1086 FORMAT([5,5X.E10.3)
 5900 FORMAT(1H1,1744/1X,3246)
 5001 FORMATC///6HNJTS = IC. 5X,6H NR = IS. 5X,6H NRE = IS. 5X,6H NPE = IS. 5X,
    17HNHODE =13.5%.6HHKEY =13.5%./HNLUMP =10)
 SOUP FORMATO////SHM A T E R I A L
                                    PROPERTIFS *********
    1 # = # # # # # # # # # # # # # # # # / / 3 科解育。
                                    YOUNG . S MODULUS
                                                      PUISSUN RATIO
    1 MODULUS OF RIGIDITY
                             DENSITY, +01/12,0x,F12,5,9X,F7.5,10X,F12.5,
    168.F12.5))
 5043 FORMATC///34HJ O [ N T
                               COORDINATES/35HJOINENO.
    1 courp.
                 Y COORD.)
 5004 FORMAT(15,7). FIR. 5,0X, F10.5)
 SOUS FORMATO///A:43 O I N T RESTRATAT
                                                    C 0 1) E ********
    100000000000000/67HJOINT NO.
                                                      ROTATION ABOUT X
                                    Z DISPLACEMENT
        ROTATION AROUT Y)
    Í
 5807 FORMAT(15,114,119,120)
 SOUR FORMATC///75HR F A M
                             ELEMENT
                                              PROPERTIES ****
    MAT
                      JOINT 1
                                 JOINT P)
    1.
 5049 FBRMAT(16,8X.F0.4.4X.FQ.4.4X.F9.4.2X,12.6X.12,9X.12)
                                        PLATE
 SOLO FORMAT(///1/2HT R I A N B U L A R
                                                      ELFMFNT
    1 PROPERTIES *********
    1****/19/MELEMENT NO.
                                Ţ
                                            JOINT 1
                                                       JOINT 2
                                                                JUINT
    13
                                          D1
              DX
                            D.Y
                                                       DXY
 5011 FORMATCI6.8 .F8.4.3X.12.6X.12.PX.12.RX.12.6X.E11.5.3X.F11.5.3X.
    1F11.5,3X,E1 7.3X,F6.2}
 5020 FORMAT(///60HCOORDINATE NUMBERS FOR FACH Z DISPLACEMENT AT EACH UN
    IRESTRAINED JOINT/25HJOINT NO.
                                        COORD. NO.)
 5021 FORMAT([5, []4)
 SAPP FORMATO///PHAL II M P F D
                                 WFIGHTS, 23HJOINT NO.
                                                                   WEIG
    IHTI
 5023 FORMAT(15,6 .F10.4)
      1 V=#
C
      DISC ASSIGNMENTS
      IN="
      CALL FLGEOF (IN. IV)
      OUT=6
```

```
MILISC=7
      NITSC=8
      INTSC=9
      JUISC=10
      KDISC=1 L
      REGIN INPUT OF UATA
  100 READ([N.1000) (TITLE(]).1=1.24)
      IF (IV.NE.U) CALL EXIT
      REWIND MDISC
      REWIND NDISC
      REWIND IDISC
      REWIND JDISC
      REWIND KDISC
      WRITE(OUT, on an) (TITLE(I), I=1,24)
      READ(IN, 104)) NJTS. NR, NBE. NPE, NMODE, MKEY, NLUMP
      MUTS=NO. OF JOINTS, NR=NO. OF JOINTS WITH RESTRAINTS
C
      NHF=NO. OF BEAM ELEMENTS, NPE=NO. OF TRIANGULAR PLATE ELEMENTS
('
      NMODE=NO. OF EIGENVALUES AND EIGENVECTORS DESIRED
C
      MKFY = 1 DO NOT COMPUTE ELEMENTAL CONSISTENT MASS TERMS
ſ.
      MKFY = 2 COMPUTE ELEMENTAL CONSISTENT MASS TERMS
C
      NLUMP = NO. OF LUMPED MASSES INPUT
Ç.
      WRITE(OUT, 500)) NJTS, NR, NBE, NPE, NMODE, MKEY, NLUMP
      INPUT MATERIAL PROPERTIES
C
      READ(IN, 1041) NMAT
      NO 10 I=1, NMAT
      READ(IN, 1002) YM(I), PR(I), GE(I), DENS(I)
      YM=YOUNG*S MOD./14**K, PR=POISSON RATIO, GE=MOD. OF RIGIDITY
ť.
(:
      DENS=DENSITY
      TF(GE(1).EQ.0.) GE(1)=YM(1)/(2.*(1.+PR(1)))
      YM(1)=YM(1)*1.F6
   1" GE(1)=GE(1)*1.Fo
      WRITE(OUT, 1002) (1,YM(1),PR(1),GE(1),BENS(1),1=1,NMAT)
      DU 250 I=1.NMAT
  254 BENS(1)=DENS(1)/(32.1/4#12.)
      INPUT JOINT COORDINATES
      READ(IN, 10'3) (X(M), Y(M), M=1, NJTS)
      WRITE (OUT, 5093)
      WRITE(OUT, 'HITA) (M, X(M), Y(M), M=1, NJTS) .
      INPUT JOINT RESTRAINT CODE
C
C
      #=FREE
      1=CLAMPED
      00 12 I=1, NJTS
      NR1(1)=U
      NR2(I)=0
      NR3(1)=0
      N(())=0
      N \times (1) = 0
   12 No([)=0
      IF (NR.EQ. (1) GO TO 80
      WRITE(OUT, 1896)
      NO 11 I=1,NR
      READ([N,10']) JT,M1,M2,M3
```

```
MR1(JI)=MI
      8R2(JI)=M2
      NR3(JT)=MS
      WRITE(OUT, SHIP?) JT, Mt, M2, H3
   LI CONTINUE
   BU CONTINUE
      GENERATE COORDINATE NUMBERS FOR EACH DEGREE OF FREEDOM. II IF
C
C
      CLAMPED, NORMAL DISPLACEMENTS ARE NUMBERED FIRST
C
      NI. NZ. NO CONTAIN COORD. NUMBERS FOR EACH JOINT
C
      NRFILU = NO. OF NORMAL DISPLACEMENTS
C
      NDF = NO. OF DEGREES OF FREEDOM INCLUDING RUTATIONS
      CALL COORDN(MRI, MR2, MR3, ML, MR, M3, MJ (S, MREDU, MDF).
      WRITE(OUT, 5020)
      DO SO I=1, NUTS
      IF (NRI(I).EU.1) GO TO 50
      WRITE(OUT, 0021) [,NI(I)
   of CONTINUE
C
      INPUT LUMPED MASSES
      IF (NLUMP.EO.A) GO TO 250
      REAU(IN, 1006) ((JHASS(I), RSHASS(I)), I=1, NLUMP)
      HRITE(OUT, 1622)
      DU 751 I=1.NLUMP
      WRITE(OUT, 9024) JMASS(I).RSMASS(I)
      RSMASS(I)=RSMASS(I)/(32.1/4*12.)
  251 CONTINUE
  258 CONTINUE
      NSSTF=NDF*(NDF+1)/2
      NO 13 I=1, NSSTF
   i 3 SST! (1)=0.
      TF (NBE.EQ.") GO TO 200
      BEGIN TO GENERATE BEAM STIFFNESS TERMS
C
      WRITE(OUT, bush)
      DO 14 NM=L.NBE
      INPUT BEAM FLEMENT PROPERTIES
C
      READ(IN, 10:4) AR, XI, YJ, MAT, JTNR, JTFR
      AREAREA OF BEAM CROSS SECTION, XIEAREA MOMENT OF INERTIA,
      YJ=EFFECTIVE TORSIONAL MOMENT OF INERTIA, MAI=MATERIAL CODE
      ITHR, JTFR=JUINT NUMBERS AT ENUS ...
C
      WRITE(OUT, 5008) NM, AR, XI, YJ, MAT, JINR, JIFR'
      SET UP CODE NUMBERS
C
      NOSC(L)=N1(JTNR)
      NUSC(2)=N2(JINR)
      NOSC(3)=N3(JINR)
      NOSC(4)=N1(JTFR)
      NUSC(5)=N2(JTFR)
      NOSG(6)=N3(JTFR)
      JF (MKEY.EQ.1) GO TO 253
      SIORE INFO. FOR LATER USE
C
      WRITE(IDISC) AR, XI, YJ, MAT, JINR, JT, R, (NOSC(I), 171, 6)
  253 CONTINUE
      (ANTL)X=IX
      X2=X(JTFR)
      Y1 = Y(JTNR)
```

```
Y/=Y(JTFR)
      FLN1H=SQRT((X2-X1)++2+(Y2-Y1)++2)
      CALL TRANS(X1, X2, Y1, Y2, FLNTH, DCS)
      E=YH(HAT)
      G=GE (MAT)
      CALL BEANK (FINTH, E, G, XI, YJ, STM, DCS)
      りけ うち ドニモッペ
      IF(NOSC(K).EQ.+) GO TO 15
      I=NUSC(K)
      00 16 N±1,4
      IF (NOSC(N).FQ.") GO TO 16
      J=NUSC(N)
      If (J.LT.I) GO TO 16
      HH=(?+J+(I-1)+(2+NDF-1))/2
      SSTE (MM)=SSTF(HH)+STM(K,N)
   16 CONTINUE
   IL CONTINUE
   14 CONTINUE
  200 CONTINUE
      IF(NRE.EO. ) GO TO OUR
      REGIN TO GENERATE TRIANGULAR PLATE STIFFNESS TERMS
C
      WRITE(OUT, 'nin)
      DU 17 NM=1.NPE
      INPUT TRIANGULAR PLATE ELEMENT PROPERTIES
C
      READ(IN, 10.5) PTH, MAT, JT1, JE', JT3, NDX
C
      PTH=PLATE THICKNESS, MAT=MATERIAL CUDE,
      JII, JT2, JT1=JOINT NUMBERS AT CORNERS, ANGLE AT JT1 MUST NUT BE
C
      9" LEGREES
      NX.11Y.DI.DXY.BFTA - FLFXURAL RIGIDITY TERMS AND ANGLE OF MATERIAL
      PRINCIPAL AXES W/W TRIANGLE LUCAL AXES
      IF(NDX.EQ. 1) RFAD(IN. 1002) DX. 04.01. DXY. BETA
      IF (NDX.EQ. !) GO TO 18
      RETA=1.
      DX=(YM(MAT)*PTH**;)/(12.*(1.-PR(MAT)**2))
      DY=DX
      DI=PR(MAT) * DX
      DXY=((1.-PR(MAT))/2.)*DX
   18 46 TA#BETA/ 1/12958
      WRITE(OUT, Suit) NM, PTH, MAT, JTL, JTZ, JT3, DX, DY, DI, DXY, BETA
      SET UP COUF NUMBERS
C
      NUSC(E)=N1(JT1)
      (LTL)SM=(S)JROM
      NOSU(3)=N3(JT1)
      NUSC(4)=Ni(JT2)
      NUSC(5)=N2(JT2)
      NOSC(6)=N3(JT2)
      MOSC(/)=NI(JT3)
      NUSC(H)=N2(JT))
      NUSC(y)=N3(JT3)
      1F (MKEY.EQ.1) GO TO 254
      SIOKE INFO. FOR LATER USE
C
       WRITE(IDISC) PTH.MAT.JT1.JT2.JT3.(NUSC(I), =1,9)
```

```
254 CONTINUE
      BX1=X(JT1)
      RX7=X(JT2)
      HY1=Y(JT1)
      RY2=Y(JT2)
      YV=SORT((BXZ-RX1)++Z+(BYZ-BY1)++2)
      CALL TRANS(BX1.BX2.BY1.BY2.Y2.NCS)
      XA=UCS(2)*(X(JT3)-8X1)-UCS(1)*(Y(JT3)-BY1)
      YJ=DCS(1)*(X(JTJ)-BX1)+DCS(Z)*(Y(JTJ)-BY1)
      CALL PLATEK(Y2.X3,Y0.BX,DY,D1,DXY,BETA.DCS,PLIK)
      no 19 K=1, 1
      IF (NOSC(K). EQ. 0) 60 TO 19
      I=NOSC(K)
      00 70 N=1,4
      IF (NOSC(N).EQ.0) GO TO 20
      J=NOSC(N)
      IF(J.LT.1) GO TO 28
      MM=(2*J+(1-1)*(2*NDF-1))/2
      SSTF(MM)=SSTF(MM)+PLTK(K,N)
  211
      CONTINUE
   19 CONTINUE
   17 CONTINUE
  340 CONTINUE
      STORE FOR REDUCTION
C
      DO (1 I=1, NDF
      NS=(2*I+(I-1)*(2*NDF-1))/2
      NE=(2*NDF+(1-1)*(2*NDF-1))/2
   /1 WRITE(HDISC) (SSTF(J), J=NS, NE)
      REWIND IDISC
      00 22 1=1.NSSTF
   22 SM(1)=0.
      IF (MKEY.EQ.1) GO TO 25F
      IF(NBE.EQ. 4) GO TO 201
      GENERATE HEAR MASS MATRICES
C
      NO 23 NM=1.NBE
      REAH(IDISC) AR.XI.YJ.HAT.JTNR.JTFR.(NOSC(1), 1=1,6)
      X.t = X(JTNR)
      XY=X(JTFR)
      YI=Y(JTNR)
      YZ=Y(JTFR)
      FLN1H=SQRT((X2-X1)++2+(Y2-Y1)++2)
      CALL TRANS(X1, X2, Y1, Y2, FLNTH, DCS)
      RHO=DENS(M41)
      CALL BEANM(FLNTH. RHO, AR, XI, YJ, SMM, NUS)
       no >4 K=1,/
       1+ (NOSC(K). EQ. ( ) 60 TU 24
       I=NUSC(K)
       00 25 N=1,5
       IF(NOSC(N).EG.0) GO TO 25
       J=NOSC(N)
       IF(J.LT.I) GO TO 25
       MM=(24J+(1-1)+(2+NDF-1))//
       SH(MM)=SH(MM)+SMM(K,N)
```

```
25 CONTINUE
   24 CONTINUE
   23 CONTINUE
  2u1 CONTINUE
      IF (NPE.EQ. .) GO TO SOL
C
      GENERATE TRIANGULAR PLATE MASS MAIRICES
      DO 76 NM=1.NPE
      READ(IDISC) PTH, MAT, JT1, JT2, JT3, (NOSC(I), I=1.9)
      AX1=X(JT1)
      8x2=X(J12)
      BY1 = Y(JT1)
      (STL)Y=CYP
      Y/=SQRT((BX/-BXL)*#2+(RY?-BY1)*#2)
      CALL TRANS(BX1.BX2.BY1.BY2.Y2.DCS)
      X.=UCS(2)*(X(JT3)-BX1)-DCS(1)*(Y(JT3)-BY1)
      Ya=BCS(1)*(X(JT3)-@X1)*DCS(2)*(Y(JT3)-BY1)
      PRHU=DENS(MAT)
      CALL PLATEM(Y2.X3,Y3,PRHO,PTH,DCS,PLTH)
      no 27 K=1,9
      IF (NOSC(K).EQ. () 60 TO 27
      I=NOSC(K)
      00 78 N=1,9
      IF (NOSC(N).EQ.") GO TO 28
      J=NUSC(N)
      TF (J.LT.I) GO TO 28
      MM=(7+J+(1-1)+(2+NDF-[))//
      SM(HM)=SM(MM)+PLTM(K,N)
   28 CUNITINUE
    7 CUNITAUE
   YE CONTINUE
  3"1 CONTINUE
      STORE FOR REDUCTION
  2,5 CONTINUE
      NO 758 I=I-NEUMP
      NN=JHASS(1)
      IF(N1(NN).EU.#) BO TO 258
      NNN=N1(NN)
      NS=(2*NNN+(NNN-1)*(2*NDF-NNN))/2
      SH(NS)=SH(NS)+RSHASS(-NNN)
  258 CONTINUE
      00 19 I=1,NHF
      NS=(2+I+(1-))+(2+NDF-())//
      NE=(2*NDF+(1-1)*( *NDF-1))/2
   / UKITE(NDISC) (SM(J), J=NS, NE)
      NUMASS=NDF-NREDU
      CALL FIGENCA. VALU. TEMP, B. C. DUM·, F. IDUM4, IDTSC, JDISC, KUISC, NUISC,
     IMUISC. NDF, NMORE, NMODE, NREDU, NOMASS)
      60 10 100
      FND
```

```
FORTRAN DECK
            ASSIGNS A COORD. NO. TO FACH DEGREE OF FREEDOM AT EACH JOINE
CCOORDN
C
      NR1, NR2, NR4 = ARRAYS CONTAINING RESTRAINT INFO. FUR EACH DEGREE
C
      OF FREEDOM AT FACH JOINT (FREE=0, CLAMPED=1)
      N1.N2,N3 = COORD. NO. FOR EACH DEGREE OF FREEDOM (NURMAL
C
C
      DISPLACEMENTS ARE NUMBERED FIRST)
C
      NJTS = NO. OF JOINTS
C
      NREDU = NO. OF NORMAL DISPLACEMENTS
C
      NOF = TOTAL NO. OF DEGREES OF FREEDOM (INSLUDING ROTATIONS)
      SUBPOUTINE COORDN(NR1, NR2, NR5, NI, N2, N3, NJTS, NRFDU, MDF)
      DIMENSION NR! (50), NR2(50), NR3(50), AL(50), N2(50), Na(50)
      N()=1
      DO JO I=1.NJTS
      IF(NRL(I).FQ.1) GO TO 10
      N1(1)=N0
      NU=NO+1
   IN CONTINUE
      HRFUU=NO-1
      DO 20 1=1.NJTS
      IF(NR2(1).FO.1) GO TO 21
      N2(1)=N0
      NU=N0+1
   21 IF (NR3(1).FO.1) GO TO 20
      No(1)=NO
      NO=NO+1
   24 CUNTINUE
      NDF=NO-J
      RETURN
      END
```

```
FORTRAN DECK
            TRANSFORMATION DIRECTION COSTNES
CTRANS
C
      XI.Y1 = COORDS. OF POINT 1
C
      x2.Y2 = COORDS. OF POINT 2
C
      FL = DISTANCE RETHEEN POINTS ( AND 2
      DCS = DIRECTION COSINES OF VECTOR FROM POINT 1 10 PUINT 2
C
      SURROUTINE TRANS(X1.X2.Y1.Y2.FL.DUS)
      DIMENSION DCS(2)
      BCS(1) = (X2 - X1) \cdot FL
      DCS(2)=(Y2-Y1)/FL
      RETURN
      FND
```

```
PLANE ORTO BEAM ELEMENT STIFFNESS MATRIX IN SYSTEM COURDS.
       FORTRAN DECK
CHEANK
      FI = BEAM LENGTH
C
      E = YOUNG+S HORULUS
C
      G = MODULUS OF RIGIDITY
C
      XI = AREA MUMENT OF INERTIA
      YJ = EFFECTIVE TORSIONAL MOMENT OF INERTIA
C
C
      STH = STIFFNESS MATRIX
C
      DCS = DIRECTION COSINES
      SURROUTINE BEANK (FL.E.G.XI.YJ.STM. BUS)
      DIMENSION STHEA, 6), DCS(2)
      21=6 #X1/FL
      ファニロャソコノデレ
       STM(1,1)=1'.*Z1/(FL*FL)
       STM(2.1)=6.*Z1*UCS(2)/FL
       STM(2,2)=4,+71+DCS(2)+DCS(2)+Z2+DCS(1)+DCS(1)
       SIM(3,1)=-1.#Z" *DCS(1)/FL
       SIM(3,2)=(-4.*Z1+Z2)*BCS(1)*BCS(2)
       SIM(3,5)=4.*71*DCS(1)*DCS(1)+Z/*DCS(2)*BGS(2)
       STH(4,1)=-STH(1,1)
       SIM(4.2)=-SIM(',1)
       STM(4,5)=-STM(3,1)
       STH(4.4)=STH(1.1)
       SIM(5.1)=SIM(2.1)
       STH(5,2)=2.+71+00S(2)+0CS(2)-Z2+0CS(1)+UCS(1)
       STM(5,3)=-(7.+71+Z2)+DCS(1)+DCS(2)
       STM(5,4)=-STM(2,1)
       S(M(5,5)=STM(2,2)
       STH(6,1)=STH(3,1)
        STM(6,2)=STM(5.3)
        STM(6,3)=2.*71*0CS(1)*DCS(1)-22*BCS(2)*UCS(2)
        SIM(6,4)=-SIM(4,1)
        SIM(6,5)=SIM(3,2)
        SIM(6,6)=SIM(3.3)
        no 10 1=2.6
        N=1-1
        DO 10 J=1.N
     10 STH(J,1)=STM(1,J)
        RETURN
        END
```

```
FORTRAN BECK
            PLANE GRID BEAM ELEMENT MASS MATRIX IN SYSTEM COORDS.
CHEAMM
      FL = BEAM LENGTH
C
C
      RHO = DENSITY
C
      A = CROSS SECTIONAL AREA
C
      XI = AREA MUMENT OF INERTIA
      XJ = EFFECTIVE TORSIONAL MOMENT OF INERTIA
ſ.
r
      SHM = MASS MATRIX
      DCS = DIRECTION CUSINES
£
      SURROUTINE REAMMOFLIRHOIAIXI, YJ, SMM, DCS)
      DIMENSION SHH(4,6), BCS(2)
      7.1=RHO*A*FL
      7/=+1 ++7
      70=X1/A
      DD=21*(13./35a+(6.*Z3)/(6.*Z2))
      CC=21*(11.*ft//10.+73/(10.*ft))
      AA=21*(22/.05.+2.*Z0/15.)
      T1=21 * YJ/( 1. + A)
      RR=21*(9./'H.~(0.*Zo)/(5.*Z2.)
      80=21*(13.*+1/120.-Z3/(10.*f4))
      SS=-Z1*(Z2 318.+2)/38.)
      PP=21 # YJ/(4. # A)
      SMM((,1)=00
      SMM(2,1)=CC+DCS(2)
      SHM(2,2)=AA+DCS(2)+DCS(2)+T1+DCS(1)+DCS(1)
      SMM(3,1)=-CC*BCS(1)
      SMM(3,2)=(-AA+TT)+DGS(1)+UCS(2)
      SMM(3,3)=AA+DUS(1)+BCS(1)+TT+BCS(/)+BUS(2)
      SMM(4,1)=RR
      SHM(4,2) = QQ * RCS(2)
      SMM(4.3)=-QU#BCS(1)
      SMM(4,4)=SMM().()
      SMM(5,1)=-SMM(4,2)
      SMM(5,2)=SS*DES(1)*DCS(2)*PP*DCS(1)*DCS(1)
      SMM(5,3) = (-SS+PP) *DCS(1) *DCS(.)
      SMM(5,4)=-SMM(",1)
      SMM(5,5)=SMM(2.2)
      SHM(6,1)=-SHM(4,3)
      SMM(6,2)=SMM(5.3)
      SMM(6,3)=SS*DCS(1)*DCS(1)*PP*UCS(/)*DCS(/)
      SHH(6,4)=-SHH(0,1)
      SMM(6,5)=SMM(3,2)
      SMM(6,6)=SMM(3.3)
      NO 10 I=2.5
      N=1-1
      00 in J=1.N
   : " SMM(J.I)=SMM(I.J)
      RETURN
```

FND

```
FORTRAN DECK
CPLATEK
      THIS SUBROUTINE DETERMINES THE STIFFNESS MAIRIX OF A
C
C
      TRIANGLE PLATE ELEMENT IN SYSTEM COORDS.
C
      Y2.X3.Y3 = COORDS. OF PLATE CORNERS IN LOCAL COURDINAISS
      DX. DY. D1. DXY. BETA = FLEXURAL RIGIDITY TERMS AND ANGLE OF MATERIAL
C
£.
      PRINCIPAL AXFS N/O TRIANGLE LOCAL AXES
C
      DCS = DIRECTION COSINES
C
      PLTK = STIFFNESS MATRIX
      SUBROUTINE PLATER (YZ, X3, Y3, DX, DY, D1, DXY, BETA, UCS, PLIK)
      BIHENSION PLTK(9,9),C(0,9),CINV(9,0),P(9,9),R(9,9)
      DIMENSION T(4,9)/STIFF(9,9),DCS(2)
                                                  (R(1,1),1(1,1))
      EQUIVALENCE (P(1,:), STIFF(1,1)),
      CALL CHAT(Y2.X3,Y3,C)
      CALL HINV(C,CINV,9)
      CALL DINMAT(Y2.X3,Y3,DX,DY,D1,DXY,RETA,P)
      GALL MATMPY(P, CINV, R, 9)
      DO 10 1=2,0
      N=1-1
      NO 10 J=1,N
      ZZ1=CINV(1.J)
      ZZ?=CINV(J.I)
      CINV(1,J)=72°
      CINV(J, [)=Z21
   IP CONTINUE
      CALL MATMPY(CINV, R. STIFF, 9)
      DO 400 1=1.4
      NO 400 J=1.9
  400 T(T,J)=0.
      T(1,1)=).
      T(4,4)=1.
      T(7,7)=1.
      T(2,2)=DCS(2)
      T(3,3)=0CS(2)
      T(5,5)=DCS(2)
      T(6,6)=DCS(2)
      T(8,8)=DCS(2)
      T(9,9)=DCS(2)
      T(2,3) = -DCS(1)
      T(5,6)=-BCS(1)
      T(R,9)=-DCS(1)
      T(3,2)=DCS(1)
       T(4,5)=DCS(1)
      T(9,8)=DCS(1)
      GALL MATMPY(STIFF, T, C, Q)
      T(2,3)=BCS(1)
       T(5,6)=DCS(1)
       T(8,9)=DCS(1)
      1(3,2)=-DCS(1)
      T(6,5)=-DCS(1)
      T(9,8) = -DCS(1)
      CALL MATMPY(T,C,PLTK,9)
```

RETURN

```
FORTRAN DECK
CCHAT
      THIS SUBROUTINE FORMS THE C MATRIX RELATING THE CURNER
C
C
      DISPLACEMENTS TO THE POLYNOMIAL DEFLECTION COEFFICIENTS
C
      FOR THE TRIANGULAR PLATE ELEMENT
      Y2.X3.Y5 = COORDS. OF PLATE CURNERS IN LOCAL COURDINATES
C.
C
      C = C MATRIX
      SURROUTINE CMAT(Y2, X3, Y3,C)
      DIMENSION C(9,9)
      NO 10 1=1,4
      90 10 J=1,9
   10 C(1,J)=0.
      C(1,1)=1.
      C(2,3)=1.
      C(3,2)=-1.
      C(4,1)=1.
      C(4,3) = Y2
      C(4,6)=Y2**2
      C(4,9)=Y2++5
      C(5,3)=1.
      C(5,6)=2**YZ
      C(5,9)=3.#Y2##2
      C(6,2)=-1.
      C(6,5)=-Y2
      C(6,8)=-Y2++7
      C(7,1)=1.
      C(7,2)=X3
      C(7,3)=Y3
      C(7,4)=X3**/
      C(7,5)=X3+Ya
     C(7.6)=Y3**/
     C(1.7)=X3++3
      C(7,8)=X3+Y3++2+Y3+X3++2
      C(7,9)=Y3**3
     C(8,3)=1.
     C(8,5)=X3
     C(P,6)=2. +Ya
      C(8,8)=2.*X3*Y3+X3*#2
      C(8,9)=3.4Y3+47
     C(0,2)=-1.
      C(0,4) = -2.*X^{3}
      C(9,5)=-Y3
      C(9,7)=-3.*X3**2
      RETURN
```

END

E. inc

```
FORTRAN DECK
            MATRIX INVERSION SUBROUTINE
CHINV
      A = MATRIX TO BE INVERTED
C.
      U = INVERTED MATRIX
C
      NM = ORDER OF MATRIX (.LE.9)
C
      SURROUTINE MINV(A, U, NM)
      DIMENSION A(9,9),U(9,9)
      00 4001 [=1,NM
      no your J=1,NM
      U(1.1)=U.U
      IF (I.EQ.J) U(I,J)=).0
 9001 CONTINUE
      EPS=0.00000001
      DO 9015 1=1, NM
       K = 1
       IF (I-NM) 9021.9007,9021
 9021 IF (A(I,I)-EPS) 9885,9886,988/
 9005 IF (-A(1,1)-EPS) 9006,9006,9007
 9006 K=K+1
       00 4023 J=1,NM
       U(1,J)=U(1,J)+U(K,J)
  9823 A(1,J)=A(1,J)+A(K,J)
       60 10 9021
  9007 01V=A(1,1)
       00 4089 3=1.NM
       U(1.J)2U(1.J)/DIV
  VIA ((L. I) A = (L. I) A PHP
       00 9015 MH=1.NH
       net T=A(MM.1)
        IF (ABS(DELT)-FPS) 9015,9015,9116
  9416 IF (MM-I) 9618,9815,9810
  MM. (=L 1100 00 0110
        H(MM, J)=U(MM, J)-U(1, J)+DELT
  9011 A(MM, J)=A(MM, J)-A(1, J)+DELT
   9815 CONTINUE
        00 9033 I=1.NM
        BO 9033 J=1.NM
   (L.1)U=(L.1)A Ecne
        RETURN
        FND
```

```
FORTRAN DECK
CDINMAT
      THIS SURROUTINE DETERMINES THE DOUBLE INTEGRAL MATRIX FOR.
C
      THE K EQUATION FOR THE TRIANGULAR PLATE ELEMENT
C
      Y2. X3, Y3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES
C
      DX. DY. D1. DXY. BFTA = FLEXURAL RIGIDITY TERMS AND ANGLE OF MATERIAL
C
      PRINCIPAL AXES H/O TRIANGLE LOCAL AXES
C
C
      P = DOUBLE INTEGRAL MATRIX
      SURPOUTINE DINMAT(Y2, X3, Y3, DX, DY, U1, DXY, BETA, P)
      DIMENSION P(9,9).D(3,3)
      00 10 1=1,4
      DO 10 J=1,0
   10 P(I,J)=0.
      CALL DMAT(DX.DY,D1.DXY,BETA,D)
      Al=DBLINT(Y2.X3.Y0.0.0)
      AZ=BBLINT(YZ,X3,Y3,1,U)
      AS=PBLINT(Y2,X3,Y), 2,0)
      A4=DBLINT(Y2.X3,Y3,U,1)
      AD=DBLINT(Y2.X3.Y3.6.2)
      Ab=UBLINT(Y2.X3,Y3,1,1)
      P(4,4)=4.*B(1,1)*A1
      P(4,5) = 4.*D(1,3)*A1
      P(4,6)=4.49(1,2)*A1
      P(4,7)=12.*b(1,1)*Ac
      P(4,8)=4.*(D(1,1)*A4+D(1,2)*A2+2.*D(1,3)*(A2+A4))
      P(4.9)=12.+U(1.2)+A4
      P(5,5)=4.#B(3,3)*A1
      P(5,6) = 4.*B(3.2)*A1
      P(5,7) = 12.*P(3,1)*A2
      P(5,8)=4.*(B(3,1)*A4+B(3,2)*A2+2.*B(3,3)*(A2+A4))
      P(5,9) = 12.*P(3,2)*A4
      P(6,6)=4.*H(2,2)*A1
      P(K,7)=12.40(2,1)+A2
      P(6.8)=4.*(B(2,1)*A4+D(2,2)*A2+2.*D(2,3)*(A2+A4))
      P(6,9)=12.*D(7,2)*A4
      P(7,7)=36.*B(1,1)*As
      P(7,8)=12.*(n(1,1)+A6+D(1,2)+A3+2.*U(1,3)*(A3+A6))
      P(/,9)=36.*D(1,2)*A6
      P(8,8)=4.*(D(1,1)*A5+B(1,2)*A6+2.*D(1,3)*(A6+A5))
            +4.*(D(2,1)*A0+D(2,2)*A3+2.*B(2,3)*(A3+A4))
            +8.*(D(3,1)*A6+D(3,2)*A3+2.*D(3,3)*(A3+A6))
     1
            +8.*(U(3,1)+A>+B(3,2)*Ao+2.*B(3,3)*(A64A5))
     1
      P(8,9)=12.*(D(1,2)*A5+D(2,2)*A6+2.*D(3,2)*(A6+A5))
      P(9,9)=36.*B(2,2)*A5
      DO 20 I=1.4
      N=1+1
      DO 20 J=N, 4
   c = P(J, I) = P(I, J)
      RETURN
      END
```

```
# FORTRAN DECK
CHAIMPY
C MULTIPLIES MATRICES A AND B TO GET C, ALL OF ORDER Non Surroutine Matmpy(A,B,C,N)
DIMENSION A(9,9),A(9,9),C(9,9)
00 J0 I=1,N
00 10 J=1,N
C(I,J)=0.
00 10 K=1,N
T(C(I,J)=0.
NO 10 K=1,N
T(C(I,J)=C(I,J)+A(I,K)+B(K,J)
RETURN
FND
```

HBROUTINF DETERMINES THE FLEXURAL RIGIDITY MATRIX IN TRIANGLE LUCAL COOPDINATES

DX.DY.DI.NY.HETA = FLEXIRAL RIGIDITY TERMS AND ANGLE OF MATERIAL PRINCIPAL AXES

D = FLEXINAL RIGIDITY HATRIX IN TRIANGLE LOCAL COOPUS.

SUBROUTINE DMAT(DX.DY.DI.DXY.BETA.D)

DIMENSION D(3,3) SINAPOLITIME DETFRAINES THE FLEXIMAL RIGIDITY MATRIX IN D(1,3)=111=213+112=223+113=233 D(2,2)=171=213+112=223+123=333 D(2,2)=171=21-122=224+173=233 D(2,2)=171=21+172=227+173=232 D(2,3)=171=214+172=223+173=233 D(3,2)=171=214+172=223+173=233 D(3,2)=171=214+172=223+133=213 RETURN D(1,1)=111+211+112+221+113+211 D(1,2)=111+212+120-22+113+232 23=113 31=-2.*\$1H(BETA)*fnS(BETA) 32=-731 11=(COS(RFTA)) ••? 17=(SIN(RETA)) ••? 11=SIN(RFTA) •COS(RETA) 31=T12 221=D1+11+0Y+112 22=D1+171+6Y+122 23=D1+111+0Y+132 31=DXY+113 12=DX+121+91+122 13=DX+141+01+122 00235 01 06-05-40

```
FORTRAN DECK
CDALINT
      THIS SUBROUTINE EVALUATES THE DOUBLE INTEGRALS APPEARING IN THE
C
C
      EQUATIONS FOR K AND M FOR THE TRIANGULAR PLATE ELEMENT
C
      Y2. X3. Y3 = COORDS. OF PLATE CORNERS IN LOCAL COURDINATES
C
      MIN = POWER OF X AND Y RESPECTIVELY, PRZEMIENIECKI, PAGE 305
      FUNCTION DREINT(Y2, X3, Y3, H.N)
      DIMENSION A)(2), B1(/), P1(7), P2(7), P3(7)
      EQUIVALENCE(R1(1),P3(1))
      TF(H-1) 40,41,42
   48 P)(1)=1.8
      N3 = 6
      GO 10 43
   41 Pi(1)=-1.0
      P1(2)=1.0
      NI=I
      GO TO 43
   42 CONTINUE
      41(1)=-1.0
      A1(2)=1.0
      91(1)=-1.0
      B)(2)=1.0
      M1 = 1
      MH=H-1
      00 10 J=1,MM
      CALL PLYMP(A1,1,81,H1,P1,N1)
      1+LN=14N
      00 10 I=1, NN1
      A1(1)=P1(1)
      M1 = N1
   10 CONTINUE
   43 CONTINUE
      IF(N-1) 50,51,52
   50 P2(1)=1.0
      M2=0
      60 10 53
   51 P2(1)=-Y3+Y2
      P2(2)=Y3
      N2=1
      90 10 53
   52 CONTINUE
      41(1)=-Y3+Y2
      41(c)=Y3
      B1(1)=-Y3+Y2
      91(2)=Y3
      M1 = 1
      NN=N-1
      DO 20 J=1.NN
      CALL PLYMP(A1,1,BL,M1,P2,N2)
      NN2=N2+1
      DO 20 I=1,NNº
```

B1(1)=P2(1)

```
M1=N2
PO CONTINUE

SOITHUE

CALL PLYMP(P1,N1,P2,N2,P3,N3)

HN3=N3+1

SOI=0.

DO 30 I=1,NN3

SOL=SOL+(X3++(M+1))+Y2+(1./FLGAT(M+M+2))+ P3(I)+(1./FLGAT(N3+2-I))

SOUTHUE

Fol INT=SOL

RETURN

END
```

is.

```
FORTRAN DECK
CPLYMP
                11/55
C
                POLYNOHIAL MULTIPLY
      SURROUTINE PLYMP(A, L. U. M. C. N)
r
      G1-434L
                   9-1-64
       DIMENSION A(1).R(1).C(1)
        N = L + M
       1.1 = N + .
        00 8 1 = ....
 ès
       C(1) = 0
       12 = 1 + 1
       M2 = M +
        00 1 1 = .12
        90 1 J = 1,42
        K = 1 + J
 1
        C(K-i) = C(K-1) + A(1) \cdot B(J)
        nu s k = 'fT
        11 = K
        IF(C(K)) 4,2,3
 2
       CONTINUE
       IF(I(-1), 5, 5, 6)
N = L1 - I1
 5
       Nº = N + 1
       00.7 J = \cdots N2
       N1 = J + 11 - L
 7
       C(J) = C(N)
       RETURN
       END
```

. 1

PI

٠,

ì

Pi

ø,

ı

p,

ı

PI

PI

PI

\$

ni bi

i

rl

PI

```
FORTRAN DICK
CPLATEM
      THIS SURROUTINF DETERMINES THE MASS MATRIX OF A
1:
\mathfrak{C}
      TRIANGLE PLATE ELEMENT IN SYSTEM COURDS.
C
      Ye.x3.Yo = COORDS. OF PLATE CORNERS IN LOCAL COURDINATES
\mathfrak{C}
      PRHU = DEMSITY
C
      PIH = PLATE THICKNESS
C
      DCS = DIRECTION COSINES
C
      PLIM = MASS MATRIX
      SURROUTINE PLATEM(YZ, X3, Y3, PRHO, PTH, DCS, PLTM)
      DIMENSION PLTM(9,9),C(9,9),CINV(9,9),P(9,9),R(9,9)
      DIMENSION T(0,0), FMASS(9,9), DCS(2)
      FUUIVALENCI (P(1,1), FMASS(1,1)),
                                                    (R(1,1)\cdot I(1,1))
      CALL CMAT(Y/.X3,Y3,C)
      CALL MINV(C, CINV, 4)
      CALL UINMTH(Y2.X3,Y0,PRHO,PIH,P)
      CALL MATHPY(P,CINV,R.y)
      00 :0 1=2,4.
      N=1-1
      00 10 J=1.N
      771=CINV(1.J)
      722=C[NV(J.])
      CINV([,J)=22'
      CINV(J, I)=/2:
   IN CONTINUE
      CALL MATMPY (CINV, R, FMASS, 9)
      no 400 1=1.9
      06-400 J=1.9
  400 T(1, J)=0.
      T(1,1)=1.
      T(4,4)=1.
      1(7,7)=).
      T(7.2)=UCS(2)
      T(3,3)=BCS(2)
      T(5,5)=UCS(/)
      T(6,6)=BCS(2)
      F(8,8)=DCS(2)
      T(9.9)=BCS(,)
      T(2.3) = -DCS(1)
      T(5,6)=-BCS(1)
      T(8,9)=-DCS(1)
      1(3,2)=DCS())
      T(4,5)=BCS(1)
      T(9,8)=DCS(1)
      CALL MATMPY (FMASS, T, C, 9)
      T(2,3) = BCS(1)
      T(5,6)=DCS(1)
      T(8,9)=DCS(1)
      T(3.2)=-BUS(')
      T(6,5)=-DCS( )
      T(9,8) = -DCS(1)
      CALL MATMPY(T.C.PLTM.9)
      RETURN
      END
```

```
FORTRAN DECK
CDINHIH
      THIS SUBRUITINF DETERMINES THE DOUBLE INTEGRAL MAIRIX FOR
C
      THE TRIANGULAR PLATE M MATRIX - PRZEMIENIECKI, PAGE 304
C
È
      Ya. X3, Ya = COURDS. OF PLATE CORNERS IN LOCAL COURDINATES
C
      PRHO = DENSITY
C
      PTH = PLATE THICKNESS
C
      P = DOUBLE INTEGRAL MATRIX
      SURROUTINE DINNTH(Y2, X3, Y3, PRHO, PTH, P)
      DIMENSION P(9,9)
      P(1,1)=DBLINT(Y2,X3,Y3,0,0)
      P(2,1) = DBLINT(Y2, X3, Y3, 1, 11)
      P(2,2)=DBLINT(Y2,X3,Y3,2,0)
      P(3,1)=DBL:NT(Y2,X3,Y3,0,1)
      P(3,2)=DBLINT(Y2,X3,Y3,1,1)
      P(4,3)=DBLINT(Y2,X3,Y3,U,2)
      P(4,1) = P(2.2)
      P(4,2)=DBLINT(Y2,X3,Y3,3,4)
      P(4,3) = BBLINT(Y2, X3, Y3, 2, 1)
      P(4,4)=D8LINT(Y2, X3, Y3.4,0)
      P(5,1)=P(3.2)
      P(5,2)=P(4.3)
      P(5,3)=DBLINT(Y2,X3,Y3,1,2)
      P(5,4)=DBLINT(Y2,X3,Y4,3,L)
    . P(5,5)=DBLINT(Y2,X3,Y3,2,2)
      P(K,1)=P(3.3)
      P(6,2)=P(5.3)
      P(A,3)=DBLINT(Y2,X3,Y3,U,5)
      P(4,4)=P(5.5)
      P(6.5)=DBLINT(Y2.X3,Y3.1,3)
      P(\kappa,\kappa)=DBLINT(Y2,X3,Y3,0,0,4)
      P(7,1)=P(4.2)
      P(7,2)=P(4.4)
      P(7,3) = P(5,4)
      P(7,4)=DBLINT(Y2,X3,Y3,5,11)
      P(7,5)=BBLINT(Y2,X3,Y3,4,1)
      P(7,6)=DBLINT(Y2,X3,Y3,3,2)
      P(7,7)=DBLINT(Y2,X3,Y3,6,0)
      P(8,1)=P(5,3)+P(4,3)
      P(8,2)=P(6,4)+P(5,4)
      P(8,3)=P(6.5)+P(5.5)
      P(8,4)=P(1,6)+P(7,5)
      P(8,5)=DBLINT(Y2, x3, Y3, 2,3)+P(7,6)
      P(8,6)=DBLINT(Y2,X3,Y3.1,4)+DHLINT(Y2,X3,Y3./,1)
      P(8,7)=DBL1NT(Y2, X3, Y3,4,2)+DBL1NT(Y2, X3, Y3,5,1)
      P(8,8)=DBEINT(Y2.X3,Y3.2,4)+BBLINT(Y2,X3,Y3,4,/)
         +2.*DBLINT(Y2,X3,Y3,3,))
      P(9.1)=P(6.3)
      P(0,2)=P(0.5)
      P(9.3)=P(6.6)
      P(9.4)=DBLINT(Y2.X3,Y0,2,3)
```

P(9,5)=DBLINT(Y2,X3,Y3,1,4)

```
P(9,6)=DBLINT(Y2,X3,Y3,U,0)
P(9,7)=DBLINT(Y2,X3,Y3,3,0)
P(9,8)=UBLINT(Y2,X3,Y3,1,0)+DBLINT(Y2,X3,Y3,7,4)
P(9,9)=DBLINT(Y2,X3,Y3,U,6)
DO JO J=1,0
DO JO J=1,1
IO P(I,J)=P(I,J)+PRHO+PTH
DO 20 I=2,0
N=I-1
DO 20 J=1,N
P(J,I)=P(I,J)
P(J,I)=P(I,J)
P( CONTINUE
RETURN
FND
```

```
FORTRAN DECK
CFIGEN
            REDUCES STIFFHESS MATRIX AND INVERTS IT, REDUCES MASS MATRIX
C
            DETERMINES EIGENVALUES AND EIGENVECTORS
C
      THE ARGUMENTS ARE=
C
    A - VECTOR OF LENGTH NRDF#(NRDF+1)/2
    VALU - VECTOR OF LENGTH HEIG
C
C
      TEMP. 8. C. DUNA. - VECTORS OF LENGTH NRDF OR NHASS (SHALLER)
C
      E - MATRIX OF DIMENSION (NRDF,3)
¢
      IDUM4 - VECTOR OF LENGTH NRDF OR NHASS (SHALLER)
C
      ITAPE, JTAPE, NTAPE, NTAPE, - THESE ARE VARIOUS TAPES
      NRDF - NUMBER OF DEGREES OF FREEDOM OF THE SYSTEM
C
C
      NEIG - NUMBER OF EIGENVALUES DESIRED
C
      NVFC - NUMBER OF EIGENVECTORS DESIRED
C
      NMASS=NO. OF NORMAL DISPLACEMENTS
C
      NOMASS=NO. OF ROTATIONAL DEGREES OF FREEDOM
C
      STIFF IS ON HTAPE IN COMPACT FORM
C
      MASS IS ON NTAPE IN COMPACT FORM
      SUBROUTINE EIGEN(A, VALU, TEMP, B.C. DUM3, E, IDUM4, ITAPE, JTAPE, KTAPF,
     1NTAPE, MTAPE, MRDF, NEIG, NVEC, NMASS, NOMASS)
      DIMENSION DUM3(NRDF), 1DUM4(1), A(1), VALU(1), B(1), C(1), E(NRDF, 3),
     1 TEMP (1):
      DIMENSION (LON(50), IHIGH(50)
      INTEGER OUT
      DATA Q1/6HFLEXIB/.Q2/6HILITY /.Q3/6HMATRIX/
      DATA Q4/6HWEIGHT/,Q5/6H MATRI/,Q6/6HX
      DO 56 II=1.NMASS
      ILOW([])=1
   56 IHIGH(II)=NMASS
      OUT=6
      REWIND HTAPE
      REWIND NTAPE
      NTEMP=NMASS
      CALL DIVID(NHASS, NOMASS, MTAPE, JTAPE, ITAPE, A.R)
      CALL ZROMAK(A,B,C,BUM3.MMASS,NOMASS,ITAPE,JTAPE,MTAPE,KTAPE)
      CALL DIVID(NMASS, NOMASS, NTAPE, JTAPE, A,B)
      CALL ZROMAM(A,B,C,DUM3,NMASS,NOMASS,ITAPE,JTAPE,NTAPE,KTAPE)
  345 CONTINUE
      REWIND HTAPE
      REWIND NTAPE
      NREDU=NMASS
      NRMX=NREDU+(NREDU+1)/2
      READ(MTAPE) (A(I), I=1, NRMX)
      WRITE(OUT,5500)
 5500 FORMAT(///85HR E D U C E D
                                  UPPFR
     1STIFFNFSS
                            MATRIX)
      00 5501 I=1,NREDU
      NS=(2+1+(1-1)+(2*NREDU-1))/2
      NE=(2+NREBU+(I-1)+(2+NREBU-I))/2
      WRITE(OUT,55%2) 1,(A(J),J=NS,NE)
 5502 FORMAT(/3HROW,14,8(/9E14.5))
 5501 CONTINUE
      GALL SYMINV(A, NREBU)
      WRITE(OUT,5503)
 5503 FORMAT(///89HR E D U C E D
                                     UPPFR
                                MATRIX)
     1FI.FXIBILITY
      PUNCH 5602, ((ILON(K), IHIGH(K)), K=1; NREDU)
 5602 FORMAT (1814)
      DO 5504 I=1, NREDU
      NS=(2+1+(1-1)+(2+NREDU-1))/2
      NE=(2*NRFDU+(1-1)*(2*NREDU-1))/2
 5504 WRITE(DUT, 5502) 1, (A(J), J=NS.NF)
```

```
PUNCH 6,011, 01,02.03
 6011 FORMAT(3A6)
      DO 5507 I=1, NREDU
      11=1-1
      IF(11.EQ.0) GO TO 5508
      DO 5509 J=1, II
      NU=(2+[+(J-1)+(2+[-J))/2+(J-1)+(NREDU-[)
 5509 B(J)=A(NU)
 5508 CONTINUE
      NS=(2+I+(I-1)+(2+NREDU-I))/2
      NE=(?*NREDU+(1-1)*(2*NREDU+1))/2
      J=1
      DO 5510 JJ=NS,NE
      B(J)=A(JJ)
 5510 J=J+1
      PUNCH 6010, (R(J), J=1, NREBU)
6010 FORMAT(1P6E12.5)
5507 CONTINUE
      OPTION TO EXPAND REDUCED FLEXIBILITY MATRIX TO FULL MATRIX BY
C
      INSERTING 1 OR 2 ZERO ROWS AND COLUMNS REPRESENTING ATTACH POINTS.
C
      CODE, NCOD = 1 OPTION EXECUTED, NCOD = 0 OPTION NOT EXECUTED
C
      READ(5,560) NCOD
  560 FORMAT (16)
      IF(NCOD) 580.580.578
 570 CALL FULFL (A.NRFDU)
 580 READ(NTAPE) (A(I), I=1, NRMX)
      DO 6012 I=1, NRMX
6012 A(1)=A(1)+32.174+12.
      WRITE(OUT, 5505)
 5585 FORMAT(///79HR E D U C E D
     1H F I G H T
                     HATRIX)
      DO 5596 I=1, NREDU
      NS=(2*I+(I-1)*(2*NREDU-I))/2
      NE=(2*NREDU+(1-1)*(2*NREDU-1))/2
 5506 WRITE(OUT, 5502) [, (A(J), J=NS.NE)
      PUNCH 6011, 04,05,06
      DO 5511 I=1,NRFDU
      11=1
      IF(II.EQ.0) GO TO 5512
      DO 5513 J=1, TI
      NU=(2*I+(J-1)*(2*I-J))/2*(J-1)*(NRFDU-I)
 5513 R(J)=A(NU)
 5512 CONTINUE
      NS=(2+I+(I-1)+(2*NREDU-I))/2
      NF=(2*NREDU+(I-1)*(2*NREDU-I))/2
      J=1
      NO 5514 JJ=NS, NE
      B(J)=A(JJ)
 5514 J=J+1
      PUNCH 6010, (R(J).J=1, NREDU)
 5511 CONTINUE
      IF (NEIG. FO. U) RETURN
      NMAX=NTEMP+(NTFMP+1)/2
   30 CONTINUE
      READ IN THE MASS MATRIX
C
      REWIND NTAPE
      READ(NTAPE) (A(I), I=1, NRMX)
      REWIND NTAPE
  355 CONTINUE
      CALL EIGHAT (NTEMP, A, VALU, TEMP, R, G, DUH3, E, I DUH4, NTAPE, NTAPE, JTAPE,
     1 ITAPE, NEIG, NVEC)
                                           105
```

100 CONTINUE
DO 60 [=1.NEIG
DUM3(I)=SORT(VALU(I))/6.2831853
60 CONTINUE
WRITE(OUT.9009)
WRITE(OUT.9005) (I.DUM3(I).I=1.NEIG)
9009 FORMAT(1H1.43x.33HHERE ARE THE NATURAL FREQUENCIES
///)
9005 FORMAT(35x.29HTHE NATURAL FREQUENCY NUMBER I3.2x.2HIS F12.3.2x.
13HCPS)
9008 FORMAT(1H1.38x.43HHERE ARE THE EIGENVALUES AND EIGENVECTORS ///)
RETURN
END

```
FORTRAN DECK
CFULFL
             EXPANDS REDUCED FLEXIBILITY MATRIX BY INSERTING 1 OR 2 ZERO
             ROWS AND COLUMNS REPRESENTING ATTACH POINTS.
C
C
      THE ARGUMENTS ARE
      B()) = REDUCED FLEXIBILITY MATRIX IN COMPACT FORM
C
C
      NXC = ORDER OF REDUCED FLEX. MATRIX
      INPUT DATA REQUIRED
C
      NR = NO. OF ATTACH POINTS (1 OR 2)
C
                                                                                      7
      NNE, NHO = HASS NUMBERS OF ATTACH POINTS 1 AND 2 RESP.
C
C
      SURROUTINE FULFL(B, NXC)
                                                                                    11 0°
      DIMENSION B(1), D(1275), C(50)
                                                                                     15
      DATA 07/6HEXPAND/.08/6HED FLE/.09/6HXIBILI/.010/6HTY MAT/.011/6HRI
                                                                                     20
     1 Y
                                                                                     25
      READ(5,1)NR, NNE, NHO
                                                                                     30
    1 FORMAT (918)
                                                                                     35
      MS=NXC+NR
                                                                                     40
      MMS=MS+(MS+1)/2
                                                                                     45
      00 50 I=1, MNS
                                                                                     50
   50 D(T)=0.0
                                                                                     55
      JJJ=0
                                                                                     60
      KK = 0
                                                                                    65
      0 = UU
                                                                                     70
      DO 100 J=1, MS
                                                                                     75
      IF(J.EQ.NNE.OR.J.EQ.NWO)GO TO 99
                                                                                     80
      I=JJ+1
                                                                                     85
      112=1+NXC-J+J(1)
                                                                                     90
      KKK=J-1
                                                                                     9.5
      DO 98 JK=I,JJ
                                                                                   100
      KKK=KKK+1
                                                                                   105
      KK=KK+1
                                                                                   110
      IF (KKK.EQ.NNF.OR.KKK.EQ.NHQ)GQ TO 96
                                                                                   115
      on 10 97
                                                                                   120
   98 KK=KK+1
                                                                                   125
   97 D(KK)=B(JK)
                                                                                   130
   98 CONTINUE
                                                                                   135
      60 TO 100
                                                                                   140
   99 KK=KK+MS-J+1
                                                                                   145
      14666=666
                                                                                   150
  100 CONTINUE
                                                                                   155
      WRITE(6,2)
                                                                                   160
    2 FORMAT(///B6HU P P E R
                                  TRIANGLE-EXPA
                                                                                   165
     1 EXIBILITY
                              MATRIX)
                                                                                   170
      DO 10 I=1, MS
                                                                                   175
      NS=(2*I+(I-1)*(2*MS-I))/2
                                                                                   180
      NE = (2 * MS + (I - 1) * (2 * MS - I))/2
                                                                                   185
      WRITE(6,3)I,(N(J),J=NS,NE)
                                                                                    190
    3 FORMAT(/3HROW, 14/(9F14.5))
                                                                                    195
   10 CONTINUE
                                                                                    200
      PUNCH 4,07,08,09,010,011
                                                                                    205
      FORMAT(5A6)
                                                                                    210
      DO 20 I=1, MS.
                                                                                    215
      11=1-1
                                                                                    224
       IF(II.E0.0) GO TO 18
                                                                                    225
      no 19 J=1, 11
                                                                                    230
      NU=(2*I+(J-1)*(2*I-J))/2*(J-1)*(MS-I)
                                                                                    235
   19 C(J)=D(NU)
                                                                                    240
   18 CONTINUE
                                                                                    245
      NS=(2*I+{I-1}*(2*MS-I))/2
                                                                                    250
      NE=(2+HS+(I-1)+(2+HS-1))/2
                                                                                    255
                                                                                    260
       J = I
```

	no je 27=N2'NE
	C(J)=D(JJ)
16	J=J+1
	PUNCH 5, (C(J), J=1, MS)
5	FURMAT(1P6E12.5)
20	CONTINUE
	RETURN
	END

```
FORTRAN BECK
CRIVIR
      N=NO. OF NORMAL DISPLACEMENTS '
C
      M=NO. OF ROTATIONAL U.O.F.
      NIPE-CONTAINS STIFFNESS (OR MASS) MATRIX
Ċ
ť.
      MIPE-K12 (MIP) STORED
c
      ITPF-K1; (MI) STORED
      A- DUMMY STORAGE VECTOR. LARGER OF (N+(N+1).2 OR M+(M+1)/2)
C
      SUBROUTINE DIVID (N.M. NTPE, HTPE, ITPE, A.B)
      DIMENSION A(').B(I)
      REWIND ITPH
      REWIND NTPI
      REWIND MTPS
      MMAX=N# (N++)/2
      MMAX=##(#++)/2
      NM=N+M
      1 CN1=0
      no 10 1=1.N
      11-NH-1+1
      READ(NTPE) (R(J), J=1.11)
      19=11-M
      no co J=1, 10
      IUNI=ICNT+ .
 0
      A(TCNT)=B(J)
      101=10+1
      JCNI=0
      96 30
               J=101.11
      JCNT=JCNT+'
      B(JCNT)=B(J)
 10
      WRITE(MTPE) (B(J), J=1.M)
 10
      CONTINUE
      WRITE(ITPE) (A(J), J=1, NMAX)
      REWIND MTPH
      RIWIND ITPH
      10:0
      TUN1=#
      90 50
               1=1,M
      II=M-ICNT
      READ(NTPE) (R(J), J=1.11)
      ICNI= | CNT+ +
      00 nm J=1.11
      1D=10+1
 60
      A([U)=B(J)
 50
      CONTINUE
      RETURN
      END
```

```
FORTRAN DECK
CZROMAK
      D IS A DUMMY VECTOR WITH STORAGE N OR H (LARGER)
C
£.
      A IS A BUMMY VECTOR WITH STORAGE N=(N+1)/2 OR M=(M+1)/2 (LARGER
C
      B IS A DUMMY VECTOR WITH STORAGE N OR M (LARGER)
C
      C IS A DUMMY VECTOR WITH STURAGE N OR M (LARGER)
C
      N=NO. OF NORMAL DISPLACEMENTS
C.
      H=NO. OF ROTATIONAL D.O.F.
\mathfrak{C}
      NIPE CONTAINS KI, MATRIX
ſ,
      MIPE CONTAINS KIZ MATRIX
C
      TIPE SCRATCH TAPE
C
      KTPE STORES K19*K/2**(-1)
C
      A INITIALLY CONTAINS K22 INVERSE
(; * # *
      REDUCED STIFFNESS MATRIX IS STORED ON TIPE
      SUPROUTINE ZROMAK(A,B,C,D,N,M,NTPE,HTPE, [TPE,KTPE)
      DIMENSION A(1),B(1),C(1),D(1)
      DUIHLE PRECISION SUH, DP1, UP2
      CALL SYMINV( A.M)
      REWIND MTPF
      REWIND ITPF
      REWIND NTPF
      REWIND KIPF
      MMAX=N#(N++)/2
      MMAX=M#(M++)/2
      00 10
              KK=1.N
      REAB(MTPE) (R(1), I=1, M)
      ICNI=0
      DO 1000 1K=1, M
      JJ=1K
      .1K = 1K
      NU / 0
              M.LL=L
      ICNI=ICNT+1
 21
      C(J)=A(ICNT)
      11=11-1
      JA=M
      10=1K
      00 30 J=1,JJ
      IF (JJ.EQ. W) GO TO SU
      C(J)=A(ID)
      J4=JA-1
      111=1D+JA
 50
      CONTINUE
      SUM=0.000
      no to
              J=1, M
      nP1=B(J)
      RP?=C(J)
 04
      SUM=SUM+DP: *DP/
      n(JK) = SUH
 1888 CONTINUE
      WRITE (ITPH) (D(J),J=1,M)
      WRITE (KTP() (D(J), J=L, M)
 10
      CONTINUE
```

REWIND ITPH

```
REWIND HTPE
      REWIND NTPF
      REWIND KTPF
      READ (NTPE) (A(J), J=1, NMAX)
      TUNT=0
      00 60 KK=1,N
      READ (ITPE) (N(J), J=1, M)
      KI=KK
      no 70
              KJ=1.N
      REAB(MTPE)(C(J), J=1, M)
     KP=kJ
     IF (KP.LT.KI) BU TO
     SUM=0.000
     90 &0 ·
              KR=1.H
     DPI=D(KR)
     npo=C(KR)
     SUM=SUM +DP:+DP2
80
     ICN1=ICNT+
     SM=SUM
     ACTONT) = ACTONT) - SH
7 R
     CUNTINUE
     REWIND HTPF
6.0
     CONTINUE
     REWIND NTPE
     REWIND MTPF
     REWIND ITPE
     WRITE(ITPE) (A(I), I=1, NMAX)
     REWIND ITPE
     RETURN
     FND
```

```
FORTRAN DECK
CZROMAM
      N=NO. 'OF NORMAL DISPLACEMENTS
      M=NO. OF ROTATIONAL D.D.F.
C
C
      NTPE CONTAINS HIT MATRIX
C
      HIPE CONTAINS MIP MATRIX
C
      ITPE SCRATCH TAPE
      KIPE CONTAINS KIP+K22++(-1)
C
      REDUCED HASS MATRIX IS STORED ON ITPE
C+++
      SURROUTINE ZROMAM(A,B,C,D,N,H,NTPE,HTPE,11PE,KTPF)
      91MENSION A(1).8(1).C(1).D(1)
      DOUBLE PRECISION SUM1.SUM2.DP1.DP2.DP3
      NHASS=N
      REWIND MTPE
      REWIND ITPF
      REWIND NTPI-
      REWIND KTPF
      NMAX=N+(N++)/2
      00 10 KK=1.N
      REAU(KTPE) (A(1), L=1, M)
       ICNI=0
      00 1000 IK=1.M
      3J=1K
      JK = IK
      10 71 J=JJ.H
       ICNI=ICNT+1
   20 C(J)=4(ICNT)
      JJ=JJ-1
      JA=M
       10=1K
       10 of J=1, JJ
       IF (JJ.EQ.8) GO TO 38
      C(J)=A(ID)
       JA=JA-1
       10=10+JA
    SE CONTINUE
       SUM) = 1.00
       00 58 J=1.M
       NP1=B(J)
       0P2=C(J)
    56 SUM1=SUM1+BP: *DP2
       n(JK)=SUM1
  IREA CONTINUE
       WRITE(ITPE) (D(J), J=1, M)
    IN CONTINUE
       REWIND ITPE
       REWIND MTPF
       REWIND NTPF
       REWIND KTPE
       (XAMM, I=L, (L)A) (39TM) (BASK
       00 60 KK=1.N
```

READ(MTPE) (R(J).J=1.M)

```
READ([TPE) (9(J).J=1.M)
   00 /0 KJ=1.N
   REAU(KTPE) (C(J), J=1, M)
   SUM3 = 0.00
   SUM2=0.DA
   00 80 KR=1.M
   1121 = B(KR)
   np2=D(KR)
   DP3=C(KR)
   SUM) = SUM1 + DP1 + DP3
AN SUHZ = SUM2+HP2+HP4
   SM1 = SUMI
   SM2=SUM2
   IF (KJ.GE.KK) MM=(/*KJ+(KK-1)*(/*NMASS-KK))/2
   IF (KJ.GE.KK) A(MM)=A(MM)-SM1+SM2
   IF (NJ.LE.KK) MM=(/*KK+(KJ-1)*(2*NMASS-KJ))/2
   IF (KJ.LE.KK) A (MM) = A (MM) - SM1
79 CONTINUE
   REWIND KTPF
60 CONTINUE
   REWIND NTPH
   REWIND HTP-
   REWIND ITPI
   REWIND KIPE
   URITE(ITPE) (A(I), I=1, NHAX)
   REWIND ITPH
   RETURN
   FRD
```

```
FORTRAN DECK
CSYMINV
         IS THE UPPER TRIANGLE OF THE SYMMETRIC MATRIX TO BE INVERTED.
C
      FLFMFNTS ARE STORED ROWWISE.
C
C
      N = ORDER OF MATRIX
                                                                                S: 1
C
      PROGRAM INVERTS IN PLACE.
      SURROUTINE SYMINV(A,N)
                                                                                ٠ς.
      DIMENSION A(1)
                                                                                ; ,
      NMAX=N*(N+1)//
                                                                                <u>.</u>
چ
      A(1)=SQRT(A(1))
      DO 180 IJ=".N
  \{1\}A(\{I\})A=\{I\}A\{1\}
                                                                                G
      A(1)=1.0/A(1)
                                                                                S
      IM1=1
      IJ=N
      DO 1040 I= .N
                                                                                S
      11=1J+1
                                                                                S
      11=11
      00 200 J=1.N
                                                                                ċ
      JHI=J-I
                                                                                S
      LI=I
      1.J=J
      DO 120 L=1.1M1
                                                                                S
      A(IJ)=A(IJ)-A(LI)+A(LJ)
      1.1=L1+N-L
  128 LJ=LI+JMI
                                                                                ১
  2011 TJ=1J+1
                                                                                S
       A(11)=SORT(A(11))
       11=1
                                                                                S
       13=1
       00 500 J=1.1ML
                                                                                c,
       (IL)A*(LL)A=(IL)A
                                                                                Ś
       IF (J-IM1)350.420,420
  300 JP1=J+1
                                                                                Ş
       JL=JJ
      1.1=11
                                                                                S
       00 400 L=JP1.[M1
       JL=JL+1
       LI=LI+N-L+1
  4UH A(J1)=A(J1)+A(JL)+A(L1)
                                                                                S
                                                                                ς
  (II)A\setminus(IU)A==(IU)A 954
       1-11-IL-IL
  5#0 JJ=JJ+N-J+1
       [F(I-N)600.900.900
  648 JP1=1+1
       11=11
                                                                                 S
       00 /04 J=1P1.N
       11=11+1
  700 A(1J)=A(1J)/A(11)
  (11) A\0.1=(11) A upe
                                                                                5
                                                                                 S
 1000 IM1=1
```

11=1

```
00 2000 1=1.N
     JJ=11
     13=11
     00 1400 J=1,N
     A(IJ)=A(IJ)+A(JJ)
     1P1=J+1
     IF (JP1-N)1100,1100,1400
1100 IL=1J
     JL=JJ
     00 1240 L=JP1.N
     IL=1L+1
    JL=JL+1
1288 A(IJ)=A(IJ)+A(IL)+A(JL)
    JJ=JL+1
U1=11 000S
    RETURN
    END
```

```
FORTRAN BECK
CETGMAT
      THIS SUBROUTINF FINDS THE EIGENVALUES AND EIGENVECTORS FOR
C
C
      SYMMETRIC MASS AND STIFFHESS MATRICES.
C
      THE ARGUMENTS ARE --
        N- ORDER OF MATRICES.
C
        A- DUMMY VECTOR WITH DIMENSION IN MAIN PROGRAM OF N*(N+1)/2
C
                STORAGE FOR FIGENVALUES. MUST BE DIMENSIONED IN THE MAIN
C
                PROGRAM AS A VECTOR OF LENGTH NEIG.
C
        TEMP.B.C.U. - DUMMY VECTORS WITH DIMENSION OF N IN MAIN PROGRAM.
C
        F- DUMMY ARRAY WITH DIMENSIONS OF (N.3) IN MAIN PROGRAM.
r.
        TOUH- DUMMY INTEGER VECTOR WITH DIMENSION OF N IN MAIN PROGRAM.
C
        MIAPE- TAPE WHERE STIFFNESS MATRIX IS STURED IN COMPACT FORM.
C
        NIAPE- TAPE WHERE: MASS MATRIX IS STORED IN COMPACT FURM.
C
        JIAPE, ITAPF- SCRATCH TAPES.
C
        NEIG-
                HUMBER OF EIGENVALUES DESIRED.
C
                NUMBER OF EIGENVECTORS DESIRED. MUST BE EQUAL TO OR LESS
        NVEC-
C
                THAN NEIG.
C
      THE MASS AND STIFFNESS MATRICES ARE STORFU IN COMPACT FURM AS
C
      VECTORS. ONLY THE UPPER TRIANGLE OF THESE MATRICES(BY RUNS) IS
C
      STORED.
      SUBROUTINE EIGHAT(N,A, VALU, TEMP, 8, C, D, F, IDUM, MTAPE, NTAPE, JTAPE.
     1 ITAPE, NEIG, NVEC)
      BIMENSION 4(1). TEMP(1). VALU(1). B(1). C(1). D(1). E(N. 1). [DUM(1)
      DOUBLE PRECISION SUM. SUM1
      INTEGER OUT
      DUT=6
      REWIND ITAPE
      REWIND JTAPE
      REWIND NTAPE
      REWIND HTAPE
      M=7+N
C
      SIFP 1
C
      READ IN M BY ROWS IN COMPACTED FORM
C
      REPLACE M HY (1) TRANSPOSE, WHERE M=L+(1) TRANSPOSE
C
      CALCULATE FIRST ROW
      READ (NYAPE) (A(I), I=L.NMAX)
      REWIND NTAPE
    6 CONTINUE
      \Lambda(1) = SQRT(\Lambda(!))
      DO :0 1=2,N
   10 A(T)=A(T)/A(1)
C
     CALCULATE ALL THE OTHER ROWS
       IND=N
      00 .01 I=2.N
      IND=IND+1
      SUM=0.Do
      K_1 = 1 - 1
      80 50 JJ=1.KI
      I+\\((r-\L)*(\J-1)//+I
```

```
20 SUM=SUM+A(MJ)#A(MJ)
      A(IND)=DSQRI(A(IND)-SUM)
      TE (IND. EQ. NMAX) GO TO :UR
      SUMI = A ( INU )
      K1 = 1 + 1
      00 99 J=K1.N
      IND=IND+1
      SUM=0.00
      11=1-1
      00 60 JJ=1.11
      K=(N-JJ)+(JJ-1)/2
      K1=K+1
      ドリード+1
   AN SUM=SUM+A(KI) *A(KJ)
      A(IND)=(A(INR)-SUM)/SUM1
   99 CONTINUE
  100 CONTINUE
  1 .. I CONTINUE
     CHECK FOR SINGULAR MASS MATRIX
      90 i0/ I=1.N
      KI=(M-1)+(1-1)/2+1
      IF (A(KI). EQ. 9.) GO TO 16911
  1.2 CONTINUE
r
     THIS COMPLETES STEP )
C *
     STLP 2
     WRITE (L) TRANSPOSE ON TAPE BY COLUMNS
ſ:
     PUT (L)TRANSPOSE INTO TEMPORARY STURAGE (TEMP--A VECTOR)
C
C
     AND THEN WRITE TEMP ON TAPE
      KIAPF=NTAPF
  THE THREE
      NU 1411 J=L.N
      no 539 1=1.J
      IND= [ND+1
      M_{1} = (M-1) * (1-1)/2 + J
      TEMP(IND)=A(MI1)
  THE CONTINUE
      WRITE(KTAPF) (YEMP(JJ).JJ=1,IND)
      IND=0
  SAP CONTINUE
r
     THIS COMPLETES STEP 2
(*************
     STEP 3
C
     ((L)TRANSPOSE) INVERSE REPLACES (L)TRANSPOSE IN CORF
C
     REPLACEMENT IS DONE BY LAST COLUMN FIRST--WORKING UP THE CULUMN
ſ.
      00 41# I=1.N
      IND=([*(M++-1))/2-N
  4 (P 4(IND)=1./A(IND)
      90 499 J=2.N
      1J=(N+2)-J
      NO .90 1=2.JJ
      5/(I-ÚL)#(6-I+L+N)=NNT
      SUM=0.Do
```

```
K1=JJ-1+2
      00 458 K=K+,JJ
      IDK=IND+K
      MK=(M-K)+(K-1)/2+JJ
  45" SUM=SUM+A(IDK)+A(MK)
      LL+dn1=dn1
      101=1ND-1+1
  490 A(IND)=-SUM+A(IUI)
  499 CONTINUE
C
     END OF STEP 3
6.*
  . . . . . . . . .
C
     STEP 4
C
     U=((L)TRANSPUSE)INVERSE
C
      WRITE U ON TAPF BY ROWS
      WRITE(ITAPF) (A(1), I=L, NMAX)
C
     FINISHED WITH STEP 4
C *
      STEP 2
C
      WRITE U ON TAPE BY COLUMNS STARTING WITH THE LAST COLUMN FIRST
C
C
      PUT U (LAST COLUMN FIRST) INTO TEMP AND THEN WRITE ON TAPE
      00 555 K=L-N
      J=N-K+1
      00 554 [=).J
      IND=IND+1
      H17=(M-1)+(1-1)/2+J
      TEMP(IND)=A(M12)
  5 in CONTINUE
      WRITE(JTAPF) (TEMP(JJ), JJ=1, IND)
      IND=0
  555 CONTINUE
     END OF STEP 5
C
f; #
   * * * * * *
     STEP 6
C
C
     FORM KU
     READ K INTO LORF
C
     READ U INTO CORF A COLUMN AT A TIME IN REVERSE ORDER
C
     REPLACE K BY KU COLUMN BY COLUMN STARTING WITH THE LAST COLUMN
C
     AND WORKING UP THE COLUMN
C
      REAU(MTAPE) (A(1), I=1, NMAX)
      REWIND JTAPE
      No 698 JJ= ,N
      J=N+1-JJ
      READ(JTAPE) (JFMP(11).11=L.J)
      BO 59# 11=1.4
       [=J+1-]]
      SUM=0.D#
      NO 650 K=1.1
      MK1=(M-K)*(K-1)/2+3
  658 SUM=SUM+A(MK1)+TEMP(K)
       IND=(M-1)+(1-1)/2+J
       IF (1.EQ.J) GO TU n8"
```

```
K1 = (M-I) + (I-1)/2
      1=1+1
      90 668 K=1.J
      KIK=K1+K
  600 SUM=SUM+A(KIK)+TEMP(K)
  680 CONTINUE
      A(IND)=SUH
  698 CONTINUE
     END OF STEP 6
C
C*
     STEP 7
C
r
      FORM((L)INVERSE) *KU
C
      KU IS IN CORF
      READ IN L COLUMN BY COLUMN AND CALCULATE ((L)INVERSE)*KU
ſ.
C
      ROW BY ROW
     CALCULATE THE FIRST ROW
C
      REWIND NTAPE
      READ(NTAPE) TEMP(1.)
      00 /10 [=1.N
  710 A(1)=A(1)/THMP(1)
      NOW CALCULAIF THE REST OF THE ROWS
      IND=N
      DO /99 I=2.N
      REAU (NTAPE) (TEMP(JJ).JJ=1.1)
      00 /99 J=1.N
      IND=IND+1
      JJ=1-L
      SUM=0.Da
      00 /5% K=1.JJ
      MK9 = (M-K) + (K-1)/2 + J
  758 SUM=SUM+TEMP(K)*A(MK2)
  7yo A(IND)=(A(IND)-SUM)/TEMP(1)
C
     STEP 7 IS COMPLETE
C.
     STEP 8
     DETERMINE EIGENVALUES AND EIGENVECTORS OF THE NEW MATRIX
C
C
      CHANGE THE SIGN OF A IN ORDER TO UBTAIN THE SMALLEST
      FIGHNVALUE FIRST
C
      DO MOU I=1.NMAX
  Bun A(1)=-A(1)
      CALL BIGMAT(A, VALU. TEMP, B, C, D, E, IDUM, N, NEIG, NVEC. MTAPE)
r
      CHANGE VALU BACK
      DO 850 I=1.NFIG
  858 VALU(I)=-VALU(I)
     STEP 8 IS COMPLETE
C
r
      STFP 0
ſ
     CHANGE EIGENVECTORS BACK
     READ U INTO CORF BY ROWS
C
     READ UNCHANGED FIGENVECTORS INTO CURE ONE AT A TIME
C
      CHANGE AND PRINT EIGENVECTORS
      IF (NVEC.EQ. ii) GO TO 2011
```

WRITE(OUT, 1091)

```
REHIND ITAPE
      REAU(ITAPE) (A(I), I=1, NMAX)
      REWIND MTAPF
      NO YOU JJ= . NYEC
      READ(MTAPE) (TEMP(I), I=1,N)
      IND=0
      00 910 F=L.N
      SUM=0.DU
      00 Y89 J=1.N
      IND=IND+1
  909 SUM=SUM+A(IND)+TEMP(J)
  910 TEMP(1)=SUM
C
      NORMALIZE THE FIGENVECTOR
      SUM=TEMP(1)
      NO 939 []= '.N
      IF (ABS(SUM)-ABS(TEMP(II))) 938,939,939
  QUE SUM=TEMP(11)
  939 CONTINUE
      IF (SUM) 945,047,948
  94" CONTINUE
      NO 941 II= .N
      TEMP(II)=TEMP(II)/SUM
  941 CONTINUE
  947 CONTINUE
  999 WRITE(OUT, 40:10) JJ, VALU(JJ), (TEMP(I), I=1, N)
     STEP 9 IS COMPLETE
C* * * * * * * * *
      60 10 2000
 4000 FORMAT (1H), 19H EIGENVECTOR NUMBER 15/12X, 17H CORRESPONDING TO
     11PF:5.7/(1H 1PAE15./)
 4881 FORMATCIHI. 38X. 434HERE ARE THE EIGENVALUES AND EIGENVECTORS
 4002 FORMAT(1H1.38X.27HTHE MASS MATRIX IS SINGULAR
 1898 WRITE(OUT. +6 2)
 2000 RETURN
      ENR
```

```
FORTRAN DECK
CHIGHAT
C PROG.AUTHORS M.ELSON AND R.E.FUNDERLIC.CENTRAL DATA PROCESSING, 4, 1, 00 6
      SURROUTINE HIGHAT(A, VALU, VALE, UPPERU, DIAG, V, T, INTER, NN, NEIG, NVFL,
     IMTAPE)
      DIMENSION A(1), VALU(1), VALL(1), UPPFRD(1), DIAG(1), V(1), T(NN, 3).
     1 INTER(1)
      REWIND MTAPE
      NZ=U
                                                                                 Н
      N=NN
                                                                                 11
      IF (N.LF.2)60 TO 49
      NP1=N+1
      NM1=N-1
      NM7=N-2
                                                                                 H
      N12P1=N+2++
      1 X = 1.
      DO 10 I=1.NM2
      SIGMA2=#.
                                                                                 н
      191:1+1
      NO 3 J=IPL.N
      1J=1X+J
                                                                                 к
    1 SIGMAZ=SIGMAZ+A(IJ)**/
      SIGHA=SORT(SIGHA?)
      II = IX + I
                                                                                 Н
      DIAG(I) = A(II)
                                                                                 K
      11P:=1X+1+1
                                                                                 н
      UPPFRD(I)=-SIGN(SIGNA, A(IIP1))
      T(I,2)=SIGMA?
      THE CARSES IGMAILED FOR THE CARREST CALLED TO 10 2
      HPPERD(1)=A(11P1)
                                                                                 н
      A(11P1)=0.
      GU 10 11
    / A(TIP1)=SQRT().+AHS(A(TIP1))/STGMA)
                                                                                 ß
      SUTGAM=-SIGN(SIGMA*A(IIP1), UPPFRD(I))
                                                                                 В
      1P2=1+2
                                                                                 Н
      00 3 J=IP2.N
                                                                                 ĸ
      1J=1X+J
   3 A(TJ)=A(TJ)/SQTGAM
      JK1=I*(2*N-I-1)/2
      JX=JKL
      TIX=JK1
                                                                                 15
      00 5 J=1P1.N
                                                                                 B
      VALL(J)=0.
      JK = JKI + J
                                                                                 H
      00 4 K=1P1.J
      1K = 1X + K
      VALL(J)=VALL(J)+A(JK)+A(IK)
   4 JK=JK+N-K
                                                                                 H
      TF(J.EO.N)GO TO 6
      CALL LOOPI (J:2.NP). VALE (J).A(JX).A(IX))
   5 JX=JX+N-J
    4 DELGAM=0.
                                                                                 Ь
      DO / J=IPL.N
```

```
[J=1X+J
 7 DEIGAM=DELGAM+A(IJ)*VALL(J)
   DGOY=.5+DELGAM
   00 b J=1P1.N
   1J=1X+J
8 T(J,1)=VALL(J)-UG02*A(IJ)
   no 4 II=IP',N
   111=1X+11
   GAIL LOOP2(A(11X),A(1X),T(NZ,L),T(11,1),A(111),1(+1,NP1)
0 11X=11X+N-11
10 IX=1X+N-I
   M=N*(N+1)/"
   UPPERD(NM1) = A(M-1)
   T(NM1,2)=UPPFRD(NM1)##?
   DIAG(NHI)=A(H-2)
   DIAG(N)=A(H)
   ENDRM=AMAX:(ABS(DIAG)+ABS(UPPERU),ABS(DIAG(N))+ABS(UPPFRU(NM1)))
   70 11 I=2.NM!
   ENRIMP=ABS(DIAG(I))+ABS(UPPERD(I))+ABS(UPPERD(I-1))
11 IF (ENRTHP.GT.ENORM) ENORM=ENRTHP
   DO 12 I=1, NEIG
   VALU(I)=ENORM
12 VALL(1)=-ENORM
   DO 24 I=1.NEIG
13 RUDY=.5+(VALU(1)+VALL(1))
   IF (ROOT.EU. VALL(1).OR.ROOT.EQ. VALU(1))GO TO 74
   NAGREE=U
   PH2=0.
   PM1=1.
   00 21 J=1.N
   TF(PM2.NE.".)GO TO 15
14 PH1=SIGN(1.,PM1)
   GO 10 1/
15 IF (PM1.NE.4.4) GO TO 17
16 P=-SIGN(1..PM2)
   PM2=0.
   1F(T(J~1,2)) 18,14,18
L7 P=DlAG(J)-ROOT-T(J-1,2)*PH2/PH1
   PM2=1.
18 IF(P)21,19.20
io PM2=PH1
   IF(PM2)21,20.20
20 NAGREE=NAGREE+1
21 PH1=P
   DU 23 J=I.NEIG
   IF (J.LE.NAGREE) GO TO 22
   IF(VALU(J).LF.ROOT)GO TO 13
   VALU(J)=ROOT
   60 10 23
22 VALL(J)=R001
23 CONTINUE
   60 TO 15
```

B

H

```
24 CONTINUE
   IF (NVEC.EU. u)GO TO 49
   FPSLON=ENORF#1.E-H
   COMPLI=COMPL(1)
   DO 48 I=1, NVFC
   NO /5 J=1.N
   V(J)=1.
   T(J,2)=DIAG(J)-VALU(I)
   IF (J.EQ.N)GO TO 24
   T(J,3)=UPPFRn(J)
// T(J+1,1)=UPPFRD(J)
26 T(N,3)=00
   00 29 J=1.N
   IF(ABS(T(J.2)).LT.1.E-1/)T(J.2)=EPSLON
   T(J,1) = T(J,2)
   T(J,2)=T(J,3)
   T(J,3)=0.
   IF (J.EQ.N)GO TO 30
   INTER(J)=0
   JP1=J+1
   IF (ABS(T(JP1.1)).LE.ABS(T(J,1)))GU TO 28
   INTER(J)=1
   10 27 K=1, .
   TEMP=T(J.K)
   T(J,K)=T(JPL,K)
27 T(JP1,K)=T+MP
28 THULTP=T(JP1.1)/T(J,1)
   VALL(J)=OR(INTER(J), AND(THULTP, COMPL1))
   T(JP1,2)=T(JP1.2)-TMULTP#T(J,2)
29 T(JP1,3)=T(JP1.3)-TMULTP*T(J,3)
30 TTFR=1
31 BO 32 J=1,N
   L=N+1-J
52 V(1)=(V(L)-1(L,2)+V(L+1)-1(L,3)*V(L+2))/T(L,1)
   VNORM=0.
   00 33 L=1,N
13 VNORM=VNORM+V(L)##2
   VNORM=SQRT(VNORM)
   DO 54 J=1.N
54 V(J)=V(J)/VNORM
   11 (1TER.EU.2)GO TO 34
   TTFR=2
   10 05 L=2,N
   1.M1=[-1
   TRY=VALL(LMI)
   IF (AND (TRY-1).EU.4) GO TO 35
   VIEMP=V(LM')
   V([M1)=V(L)
   V(L)=VTEMP
15 V(I,)=V(L)-VALL(LML)+V(I M1)
   60 10 31
36 IF (VNORM.ED.4.) V(1)=1.
```

ŀ

Ħ

A

н

HË

K

В

ß

Ř

R

H٠

H.

B.

Ħ.

Ĥ.

В

13:

B,

B

H

H

H

H

H

11X=(N+N-N-6)/2

DU 37 KK=1.NM?
IIP1=N-KK
HTV=0.
CALL LOOP3(UTV.A(LIX),V(NZ),IIP1+1.NP1)
CALL LOOP4(A(IIX),V(NZ),NP1,IIP1+1.UTV)
37 Lfx=IIX+IIP1-N-2
WRITE(MTAPF) (V(ICH),ICH=1,N)
48 CONTINUE
19 RETURN
END

4	FOR FRAN DECK
CLOUP	•
	SUBROUTINE LOOPL(JP2.NP1.SGAMPJ.AJX.AIX)
	DIMENSION AUX(1), AIX(1)
	BU I L#JP: NPI
1	SGAMPJ=SGAMPJ+AJX(L)*AIX(L)
	RETURN
	FND
\$	FORTRAN DECK
CLOOP	
	SURROUTINE LOOP2(AIIX, AIX, S, SI. AIII, IP1, NP1)
	DIMENSION ATTX(1), ATX(1), S(1)
	no , JJ=1P1, NP1 .
•	Alfx(JJ)=AllX(JJ)-All[*S(JJ)-S1*AlX(JJ)
	RETURN
	FND
ç	FORTRAN DECK
rioups	
	SURROUTINE LOUPS (UTV, 411X, V, 11P2, NPL)
	DIMENSION ATTX(1), V(1)
	no a J=IIP/,NP1
3	HIV=UTV+AITX(J)+V(J)
	RETURN
	END
q.	FORTRAN DECK
CLOUP	
	SURROUTINE LOOP4(AIIX.V,NPI,IIP2,UTV)
	DIMENSION ALTX(1), V(1)
	no a K=IIP', NPI
4	$V(K)=V(K)-\Lambda IIX(K)*UTV$
	RETURN
	FND

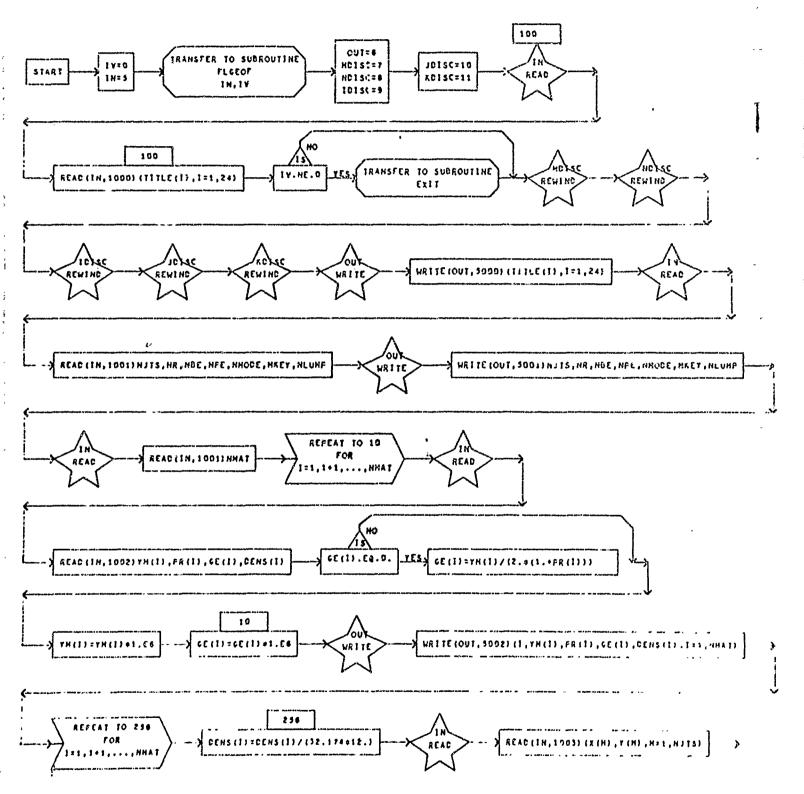
APPENDIX C

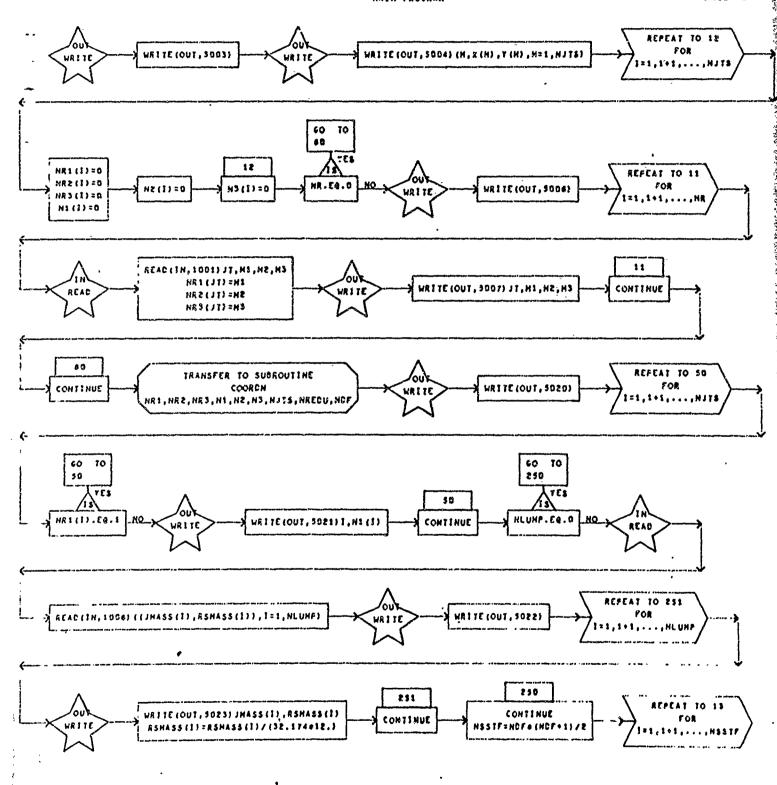
Program FLUENC FLOW C ART

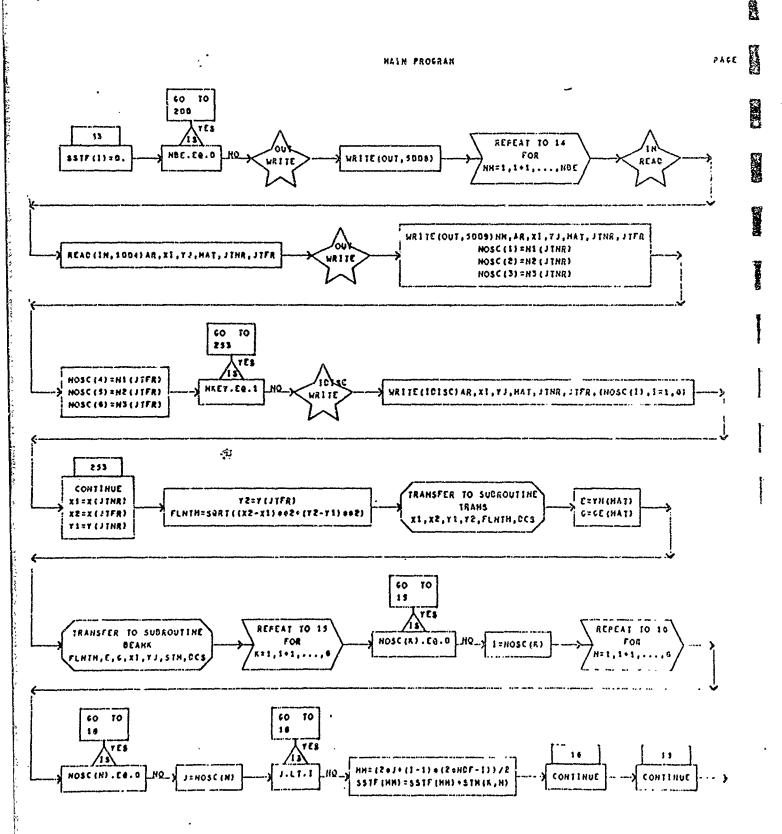
HAIN PROGRAM FLUENC-FOR GENERATING STIFFNESS, FLEXIBILITY AND MASS MATRICES FROM PLANE GRID BEAN AND TRIANG. PLATE ELEMENTS

CINENSIONES VARIABLES

SYMBOL	STORAGES	STHBOL	STORAGES	STHBOL	STORAGES	SYMBOL	STORAGES	JOEKYE	STORAGES
11 11 5	24	TM	10	PR	16	68	10	DENS	19
x	50	•	50	NR1	50	HRE	50	HR3	10
м	30	HE	50	H3	55	HOSC	<u>\$</u>	CC8	. 2
STH	6,6	SHH	*,*	PLTK	9,9	FLTH	9,9	ESTF	11325
SH	11322	RSHASS	50,A (1	25),YA	LU(9	TERP	38	•	158
c	100	CUH3	150	,	150,3	1 CUM4	50	JHASS	50

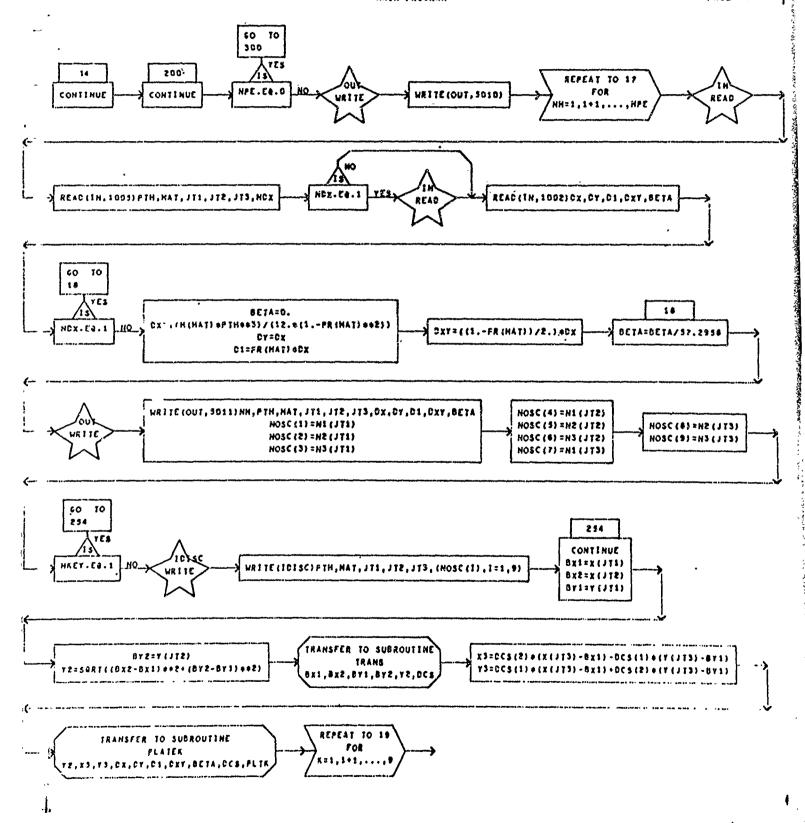


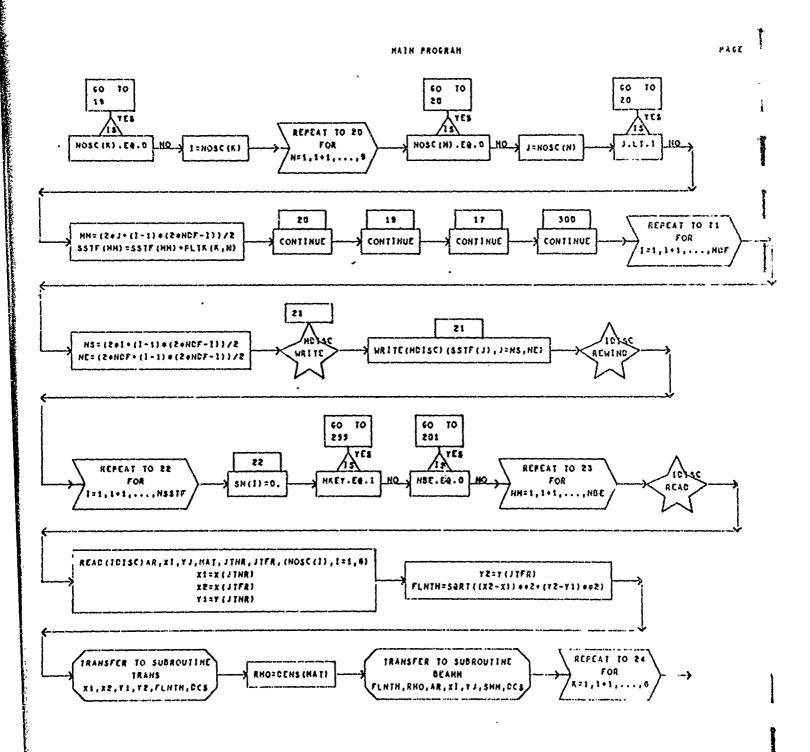


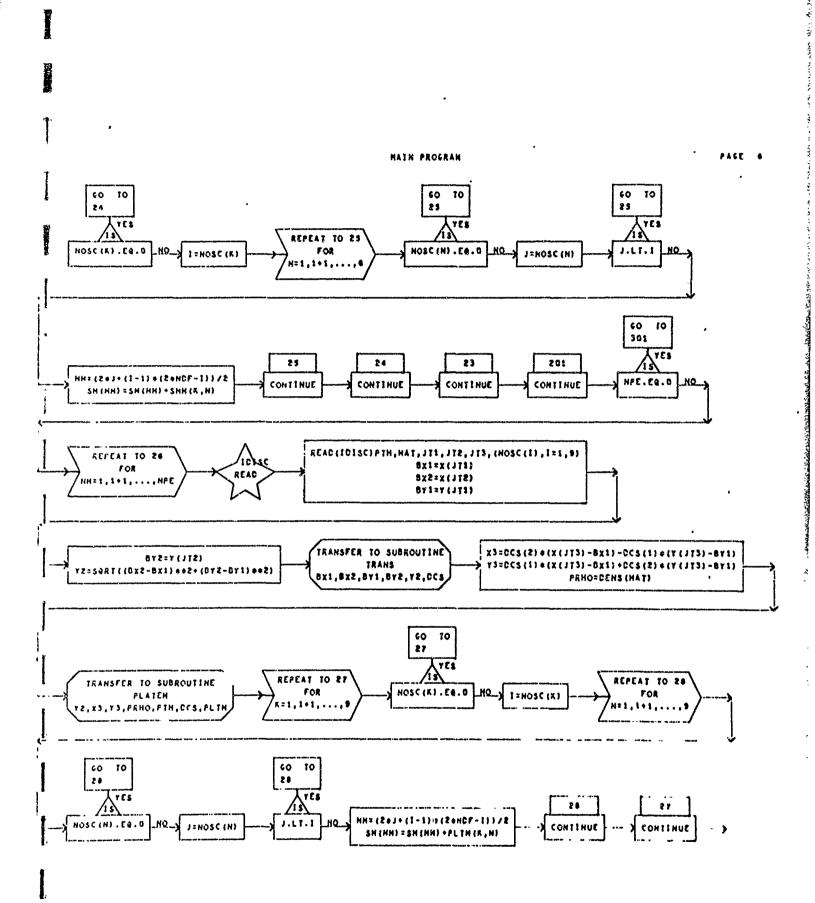


PARTIES.

Employee The Control of the Control







HAIN PROGRAH PAGE 258 TE3 255 308 REPEAT TO 258 جعانا FOR N1 (NN) . EQ. 0 CONTINUE CONTINUE CONTINUE HH= JHAS\$ (1) 258 REPEAT TO 25 нни=ні (нн) NS=(2+1+(1-1)+(2+HCF-1))/2 FOR CONTINUE ns=(2+hhh+(hnn-1)+(2+xdf-hhh))/2 11 (1-10H45) + (1-1) +70H45) = 2H SH(NS) = SH(NS) + RSHASS (HNN) 29 29 WRITE (NDISC) (SH(J), J=NS;NE) NOMASS=NCF-NRECU TRANSFER TO SUBROUTINE EIGEN 60 10 100 A, VALU, TEMP, B, C, DUM3, F, TDUM4, TDISC, JDISC, RDISC, NDISC, NDF, NHODE, NHODE, NRE CU, NOMASS

BEANK PLANE GRID BEAN ELEMENT STIFFNESS MATE IN STRICK COCRDS.

PL = BZÅH LEMGTH

E . YOUNGIS HODULUS

6 = MODULUS'OF RIGIDITY

XI = AREA MOMENT OF IMERTIA

YJ = EFFECTIVE TORSTONAL NOMERT OF INERTIA

STH = STIFFHESS MATRIX

BCS . CIRECTION COSINCE

CIMENSIONED VARIABLES

STHEOL STORAGES STORAGES STHROL STHROL STORAGES STORAGES SYMBOL STHBOL ecs. **17H** PA6E 1 SUBROUTING BEANK (FL,E,S,X1,Y),STM,8C8) STM (2, 2) = 4. +21 +DCS (2) +DCS (8) +28+DC\$ (1) +DCS (1) \$TH(4,1)=-\$TH(1,1) 23×E+X1/FL ZZ=GOTJ/FL STH(3,1)=-6.0210DC3(1)/FL STH (4,2) =-STH (2,1) START STH(1,1)=12.021/(FL0FL) STH (3,2) = (-4.021022) +DCS(1) +DCS(2) STH (4,3) = - STH (3,1) STH (2,1) = 6. 021 0CCS (2) /FL STN (3, 3) 24. +Z1+DCS(1) +DCS(1) +Z2+DCS(2) +DCS(2) STH (4,4) = STH (1,1) STH(9,1)=STH(2,1) STH(5, 5) = STH(2, 2) STH (5, 2) = 2, +21+DCS (2) +DCS (2) -22+DCS (8) +DCS (1) STH (6, 1) = STH (3, 1) STH (5, 3) =- (2. +21+22) +DCS (1) +DCS (2) STH(0,2)=STH(5,3) 57H(5,4)=-STH(8,1) \$7M(6,3)=2.021dDC\$(1)+DC\$(1)-22+DC\$(2)+DC\$(2) 10 Ľ REPEAT TO 18 REPEAT TO 10 \$TH(0,4) =-STH(5,1) Ţ FOR FOR \$1H(6,5)=\$1H(3,8) STH (J,1) *STH(1, J) 1=2,2+1,... Ų STH(0,0) = STH(3,3)

STHRIB

THIS SUBROUTINE DETERMINES THE DOUBLE INTEGRAL MATRIX FOR THE TRIANGULAR PLATE M MATRIX ~ FRZEMIENIECKI, PAGE 304

Y2,83,Y3 × COORDS. OF PLATE CORNERS IN LOCAL COORDINATES

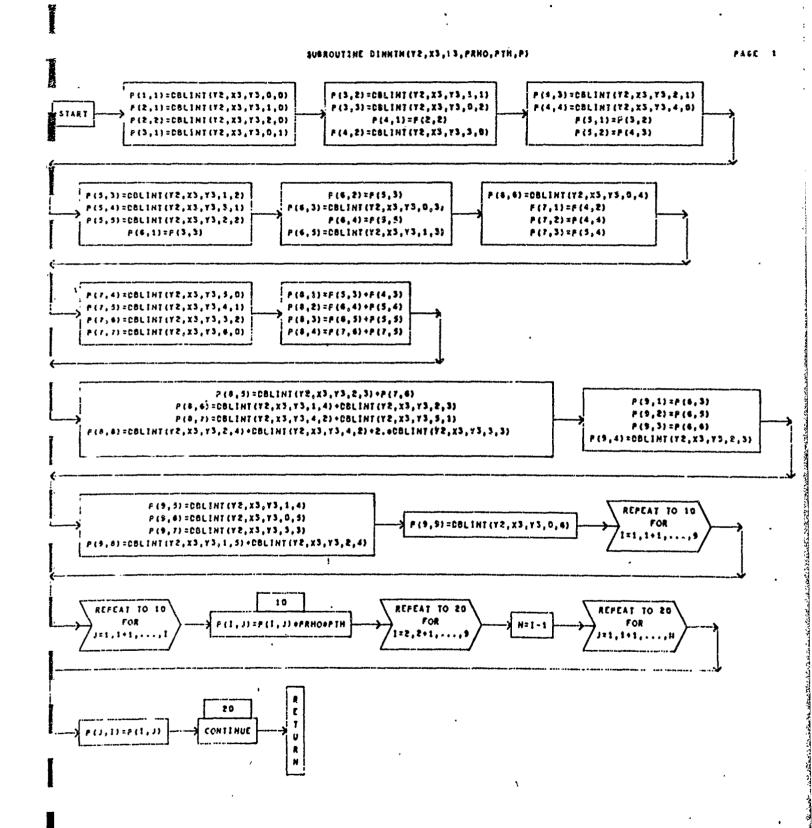
PRHO = DENSITY

PTH = PLAYE THICKNESS

P = COUBLE INTEGRAL MATRIX

EINEMBIONES VARIABLES

SYMBOL STORAGES SYMBOL STORAGES SYMBOL STORAGES SYMBOL STORAGE



MINY - MAIRIX INVERSION SUBROUTINE

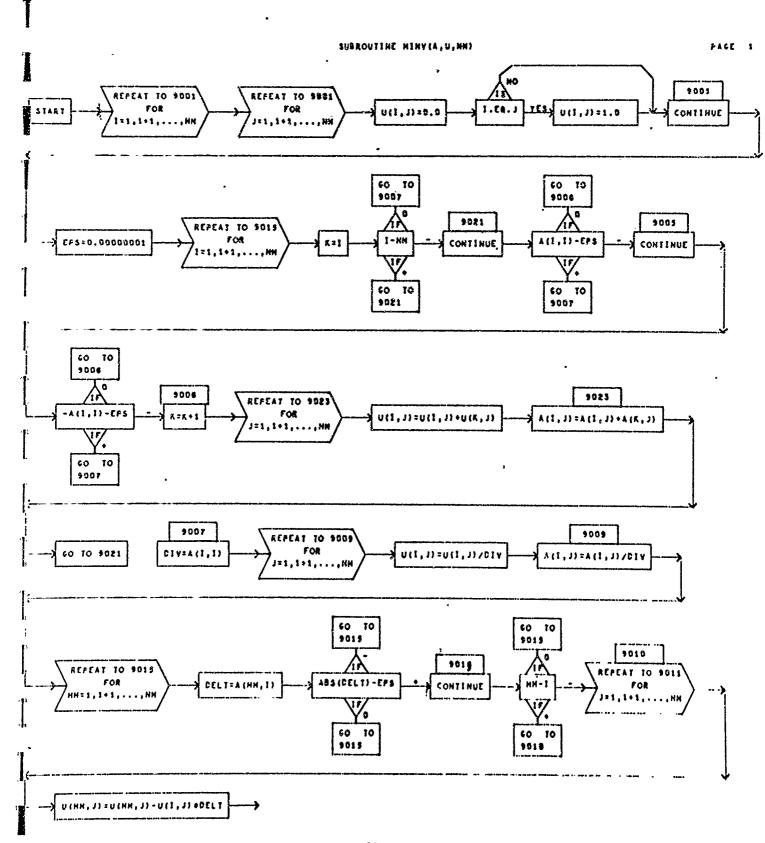
A = MAIRIX TO BE INVERTED

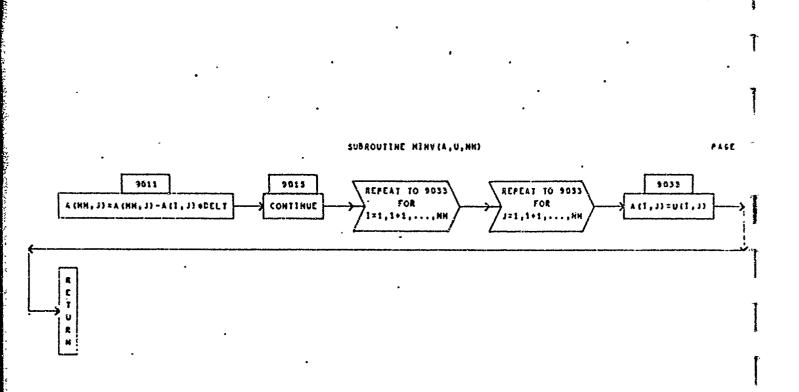
U = INVERTED MATRIX

HH = ORDER OF HATRIX (.LE.9)

BINENSIONES VARIABLES

STHROL STORAGES STHROL STORAGES STHROL STORAGES STHROL STORAGES





EIGEN REDUCES STIPPNESS HATRIX AND INVÉRTS IT, REDUCES HASS MATRIX DETERMINES EIGENVALUES AND EIGENVECTORS

THE ARGUMENTS ARE:

A - VECTOR OF LEHSTH HRBF# (HRBF+1)/8

VALU - VECTOR OF LENGTH NEIS

TEMP. B. C. CUMB. - VECTORS OF LENGTH MRDF OR HMASS (SMALLER)

E - MATRIX OF DIMENSION (MRDF, 3)

IBUM4 - YECTOR OF LENGTH HADE OR HHASS (SHALLER)

· ITAPE, JTAPE, MTAPE, MTAPE, ~ THESE ARE VARIOUS TAPES

HRCF - MUNBER OF DEGREES OF FREEDOM OF THE SYSTEM

MEIS - MUNGER OF EIGENVALUES DESTREE

MYEC - NUMBER OF ELGENVECTORS DESIRED

NHASSENG. OF NORMAL DISPLACEMENTS

MOMASSENO. OF ROTATIONAL CEGREES OF PREEDON

STIFF IS ON NTAPE IN COMPACT FORM

HASS IS ON MIAFE IN COMPACY FORM

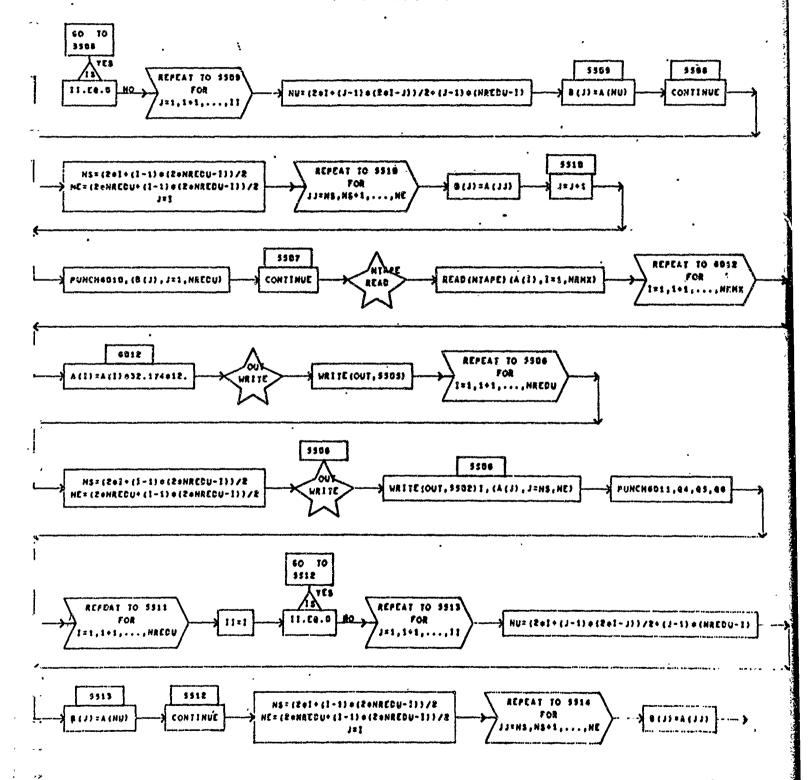
SYMBOL	STORAGES	SYMBOL.	STORAGES	STHBOL	STORAGES	SAHBOL	STORAGES	SYNGOL	STORAGES
CUM3	HRDF	16084	1	4	1	AYFA	1	₿ ,	1
•	•	e	MRDF.3	TEM	1				

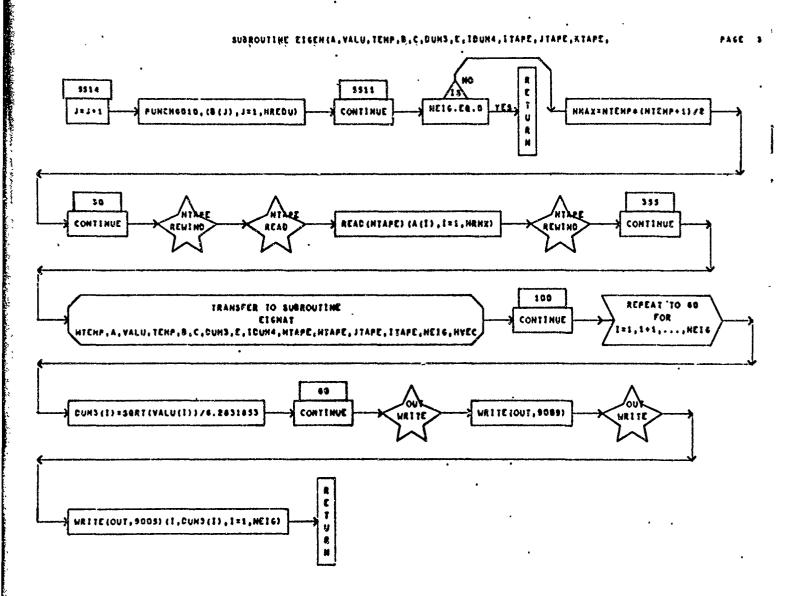
SUBGOUTINE EIGENIA, VALU, TEMP. B.C. DUNS, E. IDUNA, ITAPE, JTAPE, KTAPE, TRANSFER TO SUBROUTINE START MYENF=HMASS 00128 DIVID nhass, nohass, ntape, jtape, i tape, a , i TRANSFER TO SUBROUTINE TRANSFER TO SUBROUTINE DIVID ZROKAK MASS, NOHASS, NTAPE, JTAPE, I TAPE, A ,B,C,Duh3,Hhass,Hohass,3Tape,JTafe,Htape,Ktape 345 TRANSFER TO BURROUTINE ZRONAN CONTINUE a,b,c,dum3,nhas8,nohas3,1tafe,stape,mtafe,ktape MRECU= HMASS WRITE (OUT, 5500) READ (MTAPE) (A (1), 1=1, MRNX) HRMX=NREBU+ (NREGU+1)/2 REPEAT TO SSOL NS=(2+1+(1-1)+(2+NRECU-1))/2 FOR WRITE (OUT, 5502) 1, (A (J), J=HS, HE) HE= (2+NREGU+ (1-1)+(2+NREGU-1))/2 3381 REPEAT TO 3504 TRANSFER TO SUBROUTINE FOR SYNINY WRITE (OUT, 5503) A, WREDU 3504 5508 ᠕ NS=(2+1+(1-1)+(2+HRECU-1))/8 WRITE (OUT, 5302) 1, (A (J), J=NS, NE) PUNCH6011, 01, 42, 43 ME= (2+NRECU+ (1-1) + (2+NRECU-1))/8 REPEAT TO SSOT FOR 11:1-1

BUBROUTINE EIGEN (A. VALU, TEMP. B. C. DUNS, E. JOUNA, TAPE, JTAPE, KTAPE,

PAGE 2

William works





COORDH ASSIENS A COORS. NO. 16 TACK DESREE OF FREEDOM AT TACK JOINT

MR1, MR2, MR3 = ARRAYS CONTAINING RESTRAINT INFO. FOR EACH SESREE

OF FREEDOR AT EACH JOINT (FREE=6, CLANPED=4)

HI, HR, HI = COORD. NO. FOR EACH DEGREE OF FREEDOM (MORMAL

BISPLACEMENTS ARE NUMBERED FIRST)

HITE . NO. CF JOINTE

MREBY . NO. OF MORNAL DISPLACEMENTS

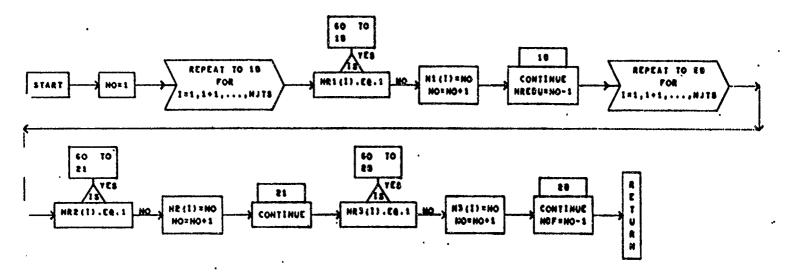
HOF . TOTAL NO. OF BEGREES OF FREESOM (ENCLUDING ROTATIONS)

SINCRESONCE VARIABLES

SYMBOL	STORAGES	STHBOL	STORAGES	SYMBOL	STORAGES	STHEOL	STORAGES	SYMBOL	STORAGES
	•		•						N.
MR1	50	HRZ	59	MRS	19	NI	10	MS	56 '
мз	50					•		•	

SUBROUTINE COORDM (MRE, MRE, MRS, ME, M2, M3, MJTS, MRESU, MBF!

PAGE 1



reline

THIS SUBROUTING EVALUATES THE SQUELE INTEGRALS APPEARING IN THE ENGALISMS FOR A AND M FOR THE TRIANGULAR PLATE ELEMENT TRIANGULAR PLATE CORRESS IN LOCAL COORSINATES MAN & POWER OF & ARB Y RESPECTIVELY, PREEMICHICAL, PAGE 385

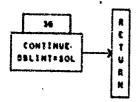
SYNBOL	STORAGES	STHBOL	STORAGES	STHEOL	STORAGES	SYNACL	STORAGES	STHROL	STORASES
41		91	,	m	7	PE	7	P2	,

P1,N1,P2,N2,P3,N3

SOL=50L+(150+(H+1))+72+(1./FLOAT(H+H+2))+P3(1)+(1./FLOAT(H5+2-1))

FUNCTION DBLINT(Y2,X3,73,K,K

PAGE >



CHAT

THIS SUBROUTINE DETERMINES THE FLEXURAL RIGIDITY HATRIX IN

TRIANGLE LOCAL COORDINATES

DX,DY,DI,DXY,BETA = FLEXURAL RIGIDITY TEXHS AND ANGLE OF NATERIAL

PRINCIPAL AXES M/O TRIANGLE LOCAL AXES

D = FLEXURAL RIGIDITY MATRIX IN TRIANGLE LOCAL COORDS.

DINENSIONES VARIABLES

STREOL STORAGES STHEOL STORAGES STREOL STORAGES STREOL STORAGES

SUBROUTINE CHAT(DX,DY,D1,CXY,BETA,B) 111= (COS (BETA)) 442 122*111 T35=T11-T12 221=01=711+07=712 112= (31H (BETA)) ++2 £17-565T Z:1=0x+T:1+01+T12 281010121.0Ye128 T13=-2.+SIN(BETA)+COS(BETA) 131=31H(BETA) +COS(BETA) 212=0x+121+01+122 223=01+131+07+132 121=112 132=-131 Z13=0x+T31+01+T32 Z31:0xY+T13 . Z32=0XY+123 0(1,3)=711+213+712+223+(130233 Ł 0(3,1)=731+211+732+221+733+231 232=0XY+133 0(2,1)=781+211+782+221+783+231 7 0(3,2)=T31+Z12+T32+Z22+T33+Z32 D(1,1)=T11+Z11+T12+Z21+T13+Z31 U 0(3,3)=731+213+732+223+733+233 9654611+8524811+812+113+238 0(2,3)=721+213+722+223+723+233

TRANS TRANSFORMATION DIRECTION COSINES

X1,Y1 = COORDS. OF POINT 1

X2,Y2 = COORDS. OF POINT 2

FL = DISTANCE BETWEEN POINTS 1 AND 2

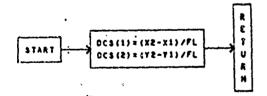
DCs = DIRECTION COSINES OF VECTOR FROM POINT 1 TO POINT 2

DINENSIONED VARIABLES

								•	
STHBOL	STORAGES	SYMBOL	STORAGES	STHBOL	STORAGES	STHBOL	STORAGES	SYMBOL	STORAGES
								!	
903	£		,					_	

SUBROUTINE TRANS (X1, X2, Y1, Y2, FL, DCS)

PAGE



BENMAS

THIS SUBROUTINE DETERMINES THE BOUBLE INTEGRAL HATRIX FOR

THE R EQUATION FOR THE TRIANGULAR PLATE ELEMENT

Y2, X3, Y3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES

DX, DY, D1, DXY, BETA = FLERURAL RIGIDITY TERMS AND ANGLE OF MATERIAL

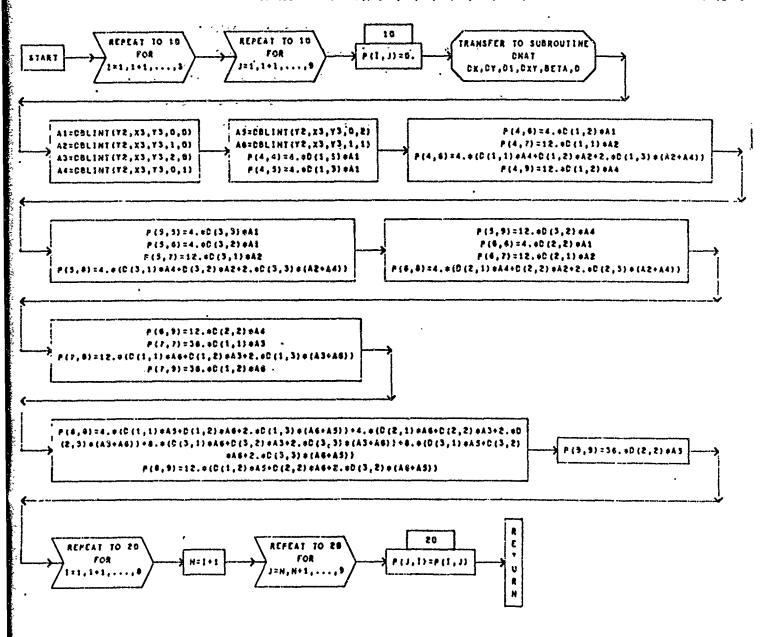
PRINCIPAL AXES W/O TRIANGLE LOCAL AXES

F = BOUBLE INTEGRAL MATRIX

BINEMAIONES VARIABLES

•	STHBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	¥104Ve£9	eyheol	STORAGES	STHOOL	STORAGES

; .



CMAT

THIS SUBROUTINE FORMS THE C HATRIX RELATING THE CORNER

DISPLACEMENTS TO THE POLYNOMIAL DEFLECTION COEFFICIENTS

FOR THE TRIANGULAR PLATE ELEMENT

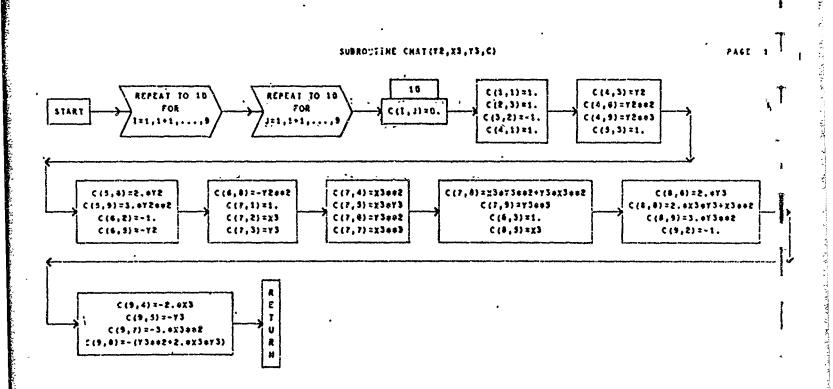
Y2, X3, Y3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES

C = C MATRIX

BIHENSIONED VARIABLES

SYMBOL STORAGES SYMBOL STORAGES SYMBOL STORAGES SYMBOL STORAGES SYMBOL STORAGES

153



National Section 1981

PLAYER

THIS SUBROUTINE DETERMINES THE STIFFHESS NATRIX OF A

TRIANGLE PLAYE ELEMENT IN SYSTEM COORDS.

Y2, X3, Y3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES

Dx.Dy,Di,Dxy,Beta = Flexural risidity terms and angle of naterial

PRINCIPAL AXES W/O TRIANGLE LOCAL AXES

DCS = DIRECTION COSINES

PLTK = STIPFHESS MATRIX

BINENSIONED . VARIABLES

SYMBOL	STORAGES	STHBOL	STORAGES .	STHEOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
PLTK	3,9	ť	. 9,9	CINV	9,9	•	•,•	R	9,9
7	9,9	811 <i>FF</i>	4,6	968	ŧ				



PAGE 1

TRANSFER TO SUBROUTINE TRANSFER TO SUBROUTINE TRANSFER TO SUBROUTINE START CHAT MINA TAHKID 75.X3.Y3.C C,CINV,D 12, X3, 73, 0X, CY, D1, CX7, BE [A, [10 221=C1NV(1,J) REPEÀT 10 10 REPEAT TO 10 TRANSFER TO SUBROUTINE SSS=CINV(),1) FOR FOR HATHPY M=1-1 CONTINUE \$\$\$=(t,1)vH1) 1=2,2+1,...,9 JE1,1+1,...,N P, CIHV, R, 9 C1HV(J.1)=ZZ1 Y(1,1)=1. REPEAT TO 400 REPEAT TO 400 TRANSFER TO SUBROUTINE T(4,4)=1. HATHET FOR FOR T(1, J)=0. 7 (7,7)=1. I=1,1+1,.. J=1,1+1,. CINV,R,STIFF,9 T(2,2)=0CS(2) 1(3,3)=0(\$(2) T(9,9)=0CS(2) 1(2,3)=005(1) T (3,2)=0CS(8) TRANSFER TO SUBROUTINE 1(5,5)=005(2) 1(2,3)=-005(1) T(5,6)=0C5(1) T(6,5)=DCS(1) HATHPY T(0,0)=CCS(2) 7(5,6)=-003(1) T(8,9)=DCS(1) \$11FF,1,C,9 T(9,6)=CCS(1) 1(8,8)=00\$(2) T(8,9) =-CC\$(1) T(3,2)=-0C5(1) R

ε

1

U

TRANSFER TO SUBROUTINE

HATHPY

T.C.PLTK.9

T(6,5)=-DC5(1)

1(9,8)=-CCS(1)

PLATEN

THIS SUBROUTINE DETERMINES THE MASS HATRIX OF A

TRIANGLE PLATE ELEMENT IN SYSTEM COORDS.

Y2, X3, Y5 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES

PRHO = DENSITY

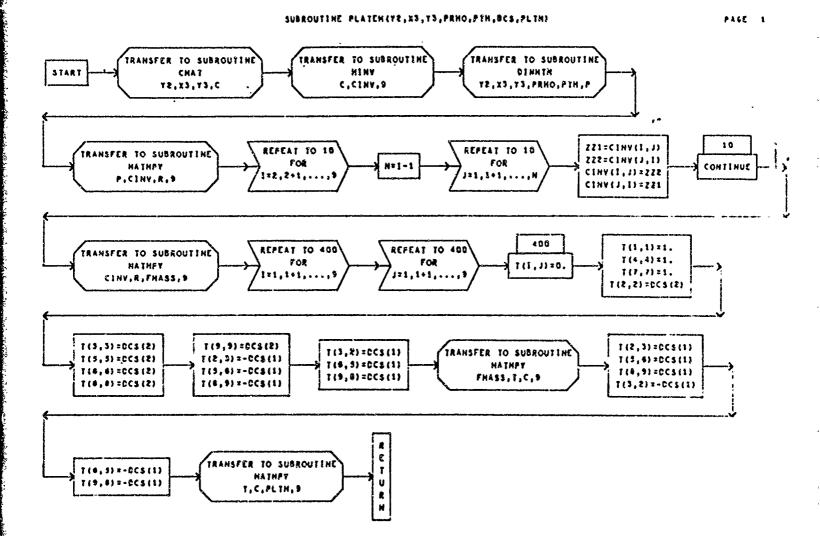
FIH = PLATE THICKNESS

DCS = DIRECTION COSINES

PLIN = HASS MATRIX

BINENSIONER VARIABLE.

STHBOL	STORAGES	SYMBOL	STORAGES	STHBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
PL TM	9,9	c	9,9	CIMA	9,0	•	9,9		•
T	*,*	FHASS	9.9	Des			• •	•	9,9



BEANN PLANE GRID DEAN ELEMENT MASS HARRIX IN SYSTEM COORDS.

FL * BEAH LENGTH

PHO = DEMSITY

A = CROSS SECTIONAL AREA

HI = AREA MONENT OF INERTIA

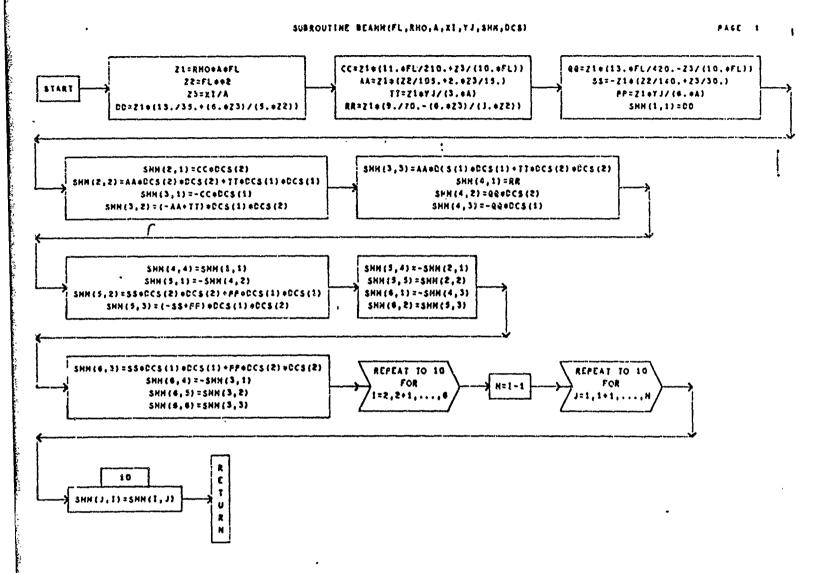
XJ = EFFECTIVE TORSTONAL MOMENT OF INERTIA

SHH = HASS HATREX

BCS = DIRECTION COSINES

BIRCHSIONES VANIABLES

STHBOL	STORAGES '	JOEHTE.	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
SAH	4,4	čcs.	ŧ						



BIAIS

MENO. OF NORMAL BISFLACEMENTS

H=NO. OF ROTATIONAL D.O.F.

HTPE-CONTAINS STIFFHESS (OR MASS) MATRIX

HIPE-KIR (HIR) STORES

ITPE-RES' (MES) STORED

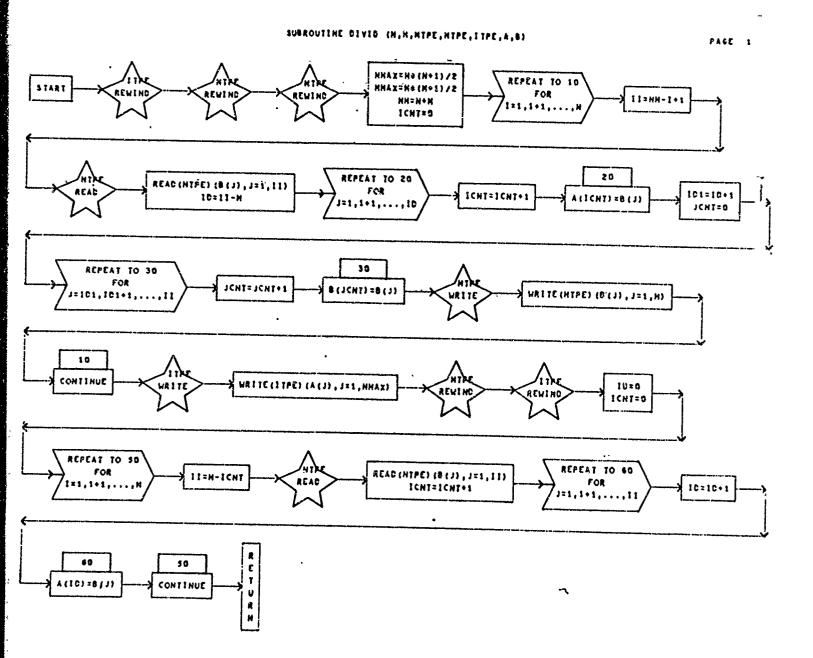
A- CUMMY STORAGE VECTOR, LARGER OF (No (N+1)/2 OR He (N+1)/2)

DIMENSIONED VARIABLES

SYMBOL STORACES SYMBOL STORACES SYMBOL STORACES SYMBOL STORACES

161

Practice.



ZROMÁK

B IS A BUNNY VECTOR WITH STORAGE N OR N (KARGER)
A IS A DUNNY VECTOR WITH STORAGE NO(N+1)/B OR N+(N+1)/B (KARGER)

B IS A DUMNY VECTOR WITH STORAGE M OR N (LARGER)

C 14 & BURNY VECTOR WITH STORAGE M OR M (LARSER)

Nº NO. OF NORMAL DISPLACEMENTS

HEND. OF ROTATIONAL B.O.F.

NIPE CONTAINS RIS HATRIX

HIFE CONTAINS KIE MATRIX

TIPE SCRATCH TAPE

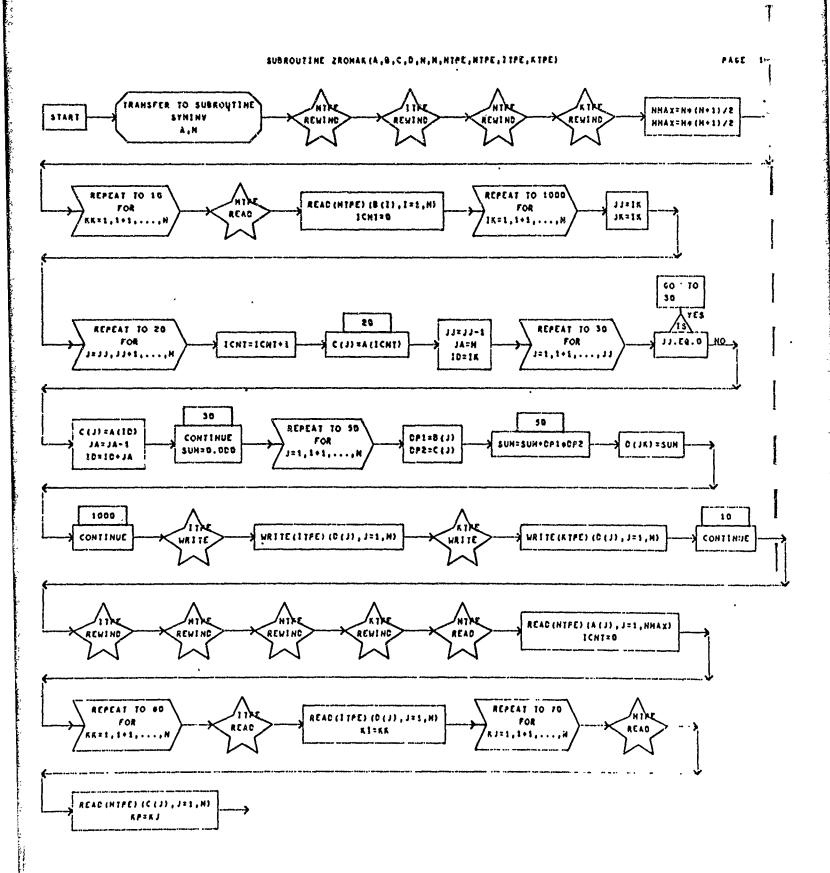
KIPE STORES KIZOKZZOG(-1)

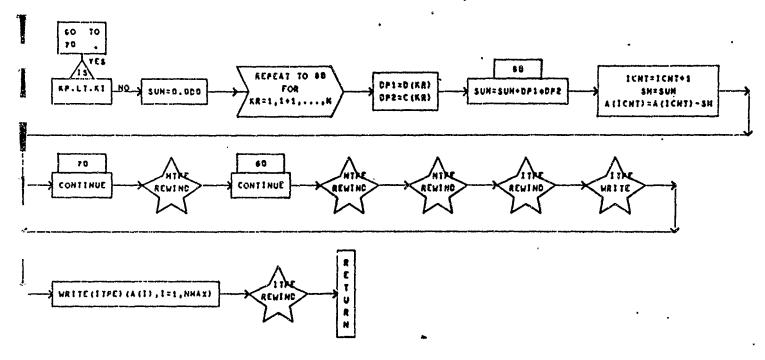
A INTITALLY CONTAINS KEE INVERSE

*** REDUCES STIFFHESS MATRIX IS STORES ON TIPE

DINENSIONED VARIABLES

SAHBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	Symbol	STORAGES	SYNBOL	STORAGES
	•		1	c	1	ð	1		





KAHORS

NENO. OF NORMAL DISPLACEMENTS

HEHO. OF ROTATIONAL D.O.F.

HTPE CONTAINS HIS HATRIX

HIPE CONTAINS HIS HAYRIX

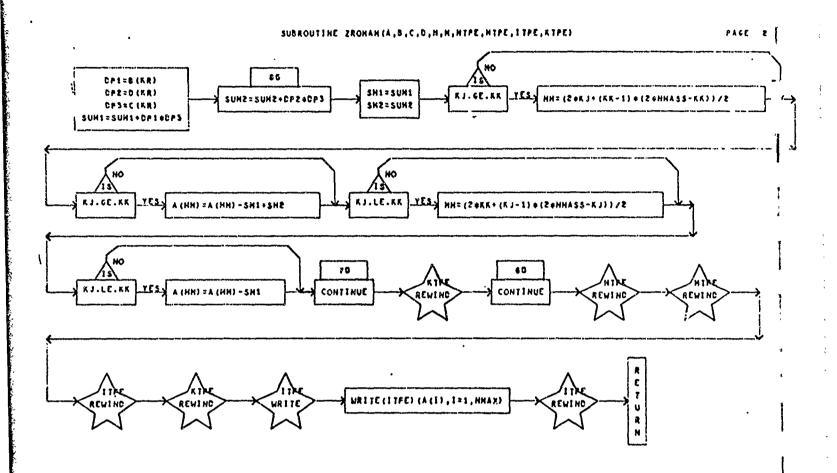
TIPE SCRATCH TAPE

KIPE CONTAINS KIZOKZZOO(-1)

*** REDUCED HASS HATRIX IS STORED ON TIPE

BINENSIONED VARIABLES

STHBOL	STORAGES	STHBOL	STORAGES	SYHBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
A	1	•	1.	c	1	0	1		1



MATMPY

HULTIFLIES MATRICES A AND 8 TO GET C. ALL OF ORDER HOM

CINENSIONES VÁRIABLES

STHBOL	STORAGES	STHBOL	STORAGES	SYMBOL	STORAGES	Synbol	STORAGES	STHEOL	\$334ROTE
A	*,*	6	9,9	¢	٠,٠				

SUBROUTINE NATHPY (A,B,C,N)

PAGE 1.

REFEAT TO 10

FOR

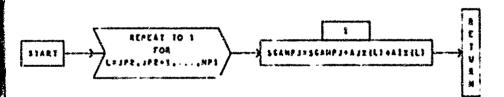
| TOR | FOR
10051

1

CINCRSIGNED VERTABLES

17HBOL	\$1083683	STREOL	310£45C4	etheol	87024654	TARGE	\$108A6E3	STREOL	\$1024628
AJR	1	AIX	1						

SURROUTING LOOPISSE, NPL, SEAMPS, 435, 415



LOOPE

CINENSIONES VARIABLES

STHSOL	STORAGES	STHBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYNBOL	STORAGES
ALIZ	1	Alx	1	\$	1				

SURBOUTING LOOPS (ALLY, ALY, A. AL, ALLI, 191, NP1)

START POR ASSECUTION A

LOOP3

DIMENSIONED VARIABLES

SYMBOL	STORAGES	SYHBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	STHBOL	STORAGES
ATTY	1	•							

SUBROUTINE LOOPS (UTV, Allx, V, 11P2, NP1)

REPEAT TO 3
FOR
J=IIP2, IIP2+1,..., HP1

REPEAT TO 3
UTV=UTV+AIIX(J)+V(J)
R
N

172

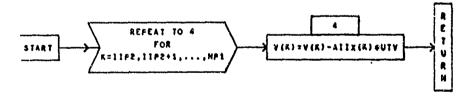
LOOP4

BINCHSIONED VARIABLES

STHBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	8; ORAGES	SYMBOL	STORAGES	SYMBOL	\$102A6E\$
					•				
A71X	3	٧	1					•	•

SUBROUTINE LOOP4 (AIIX, V, MP1, IIPE, UTV)

PAGE



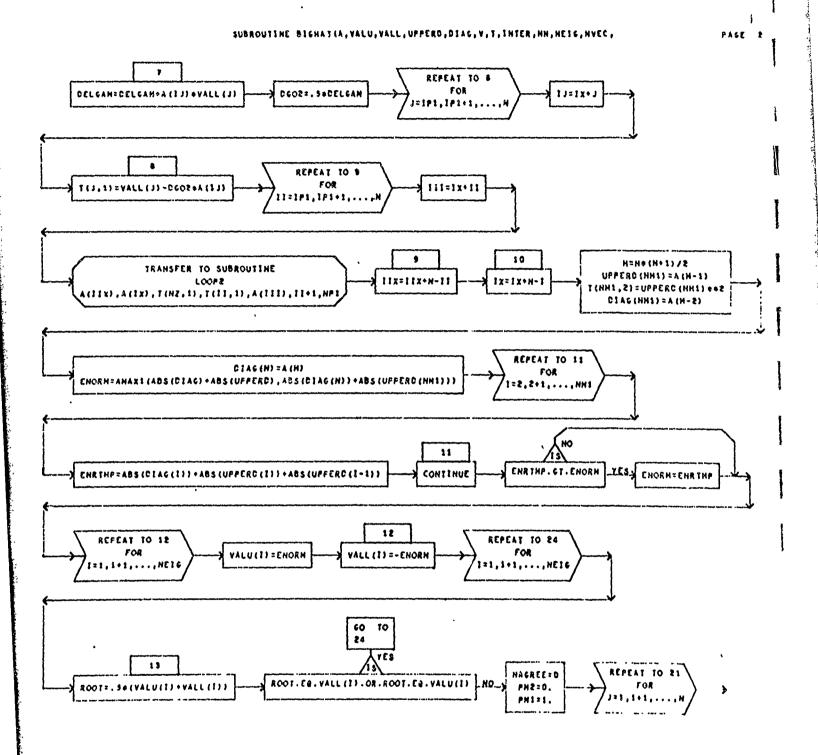
~ BI GHA!

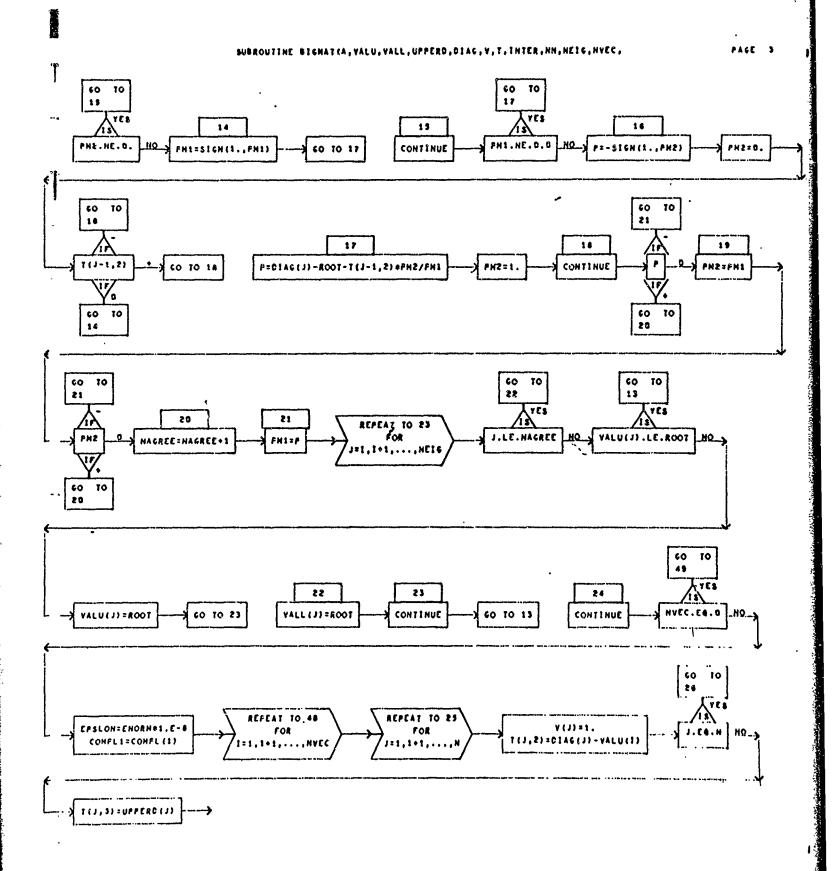
FROG. AUTHORS M. ELSON AND R.E. FUNDERLIC, CENTRAL DATA PROCESSING, 4, 2, 65

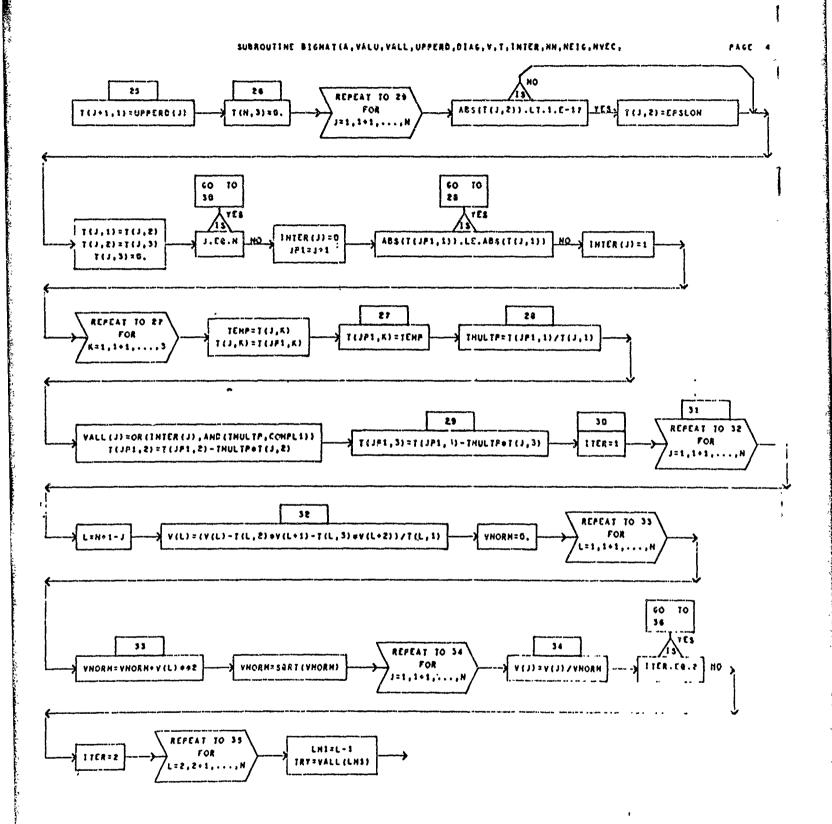
DINENSIONED VARIABLES

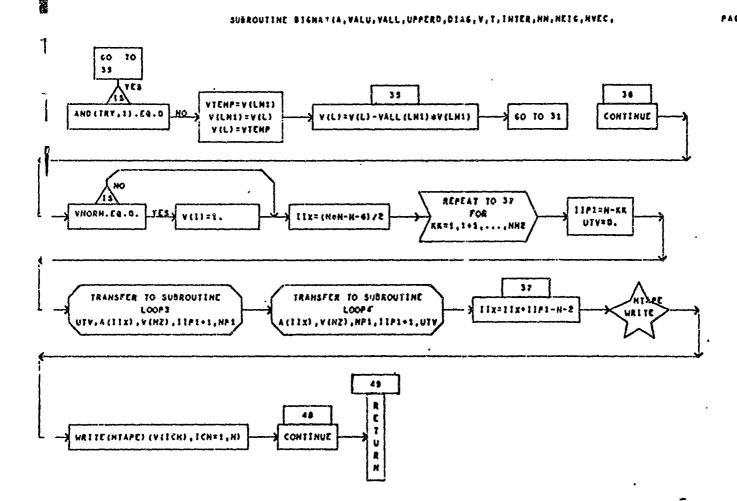
STHBOL	STORAGES.	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	SYORAGES	SYMBOL	STORAGES
A	1	YALU	1	VALL	1	UPPERO	t	CIAG	t
٧	1	T	NN, 3	INTER	1				

SUBROUTINE BIGHATIA, VALU, VALL, UPPERD, GIAG, V. T., INTER, HN, HEIG, HVEC, PACE 1 60 10 49 TYES. NP1=N+1 REPEAT TO 15 SIGNAR:0. 0 = \$K. HH1=H-1 H.LE.2 FOR IXEO START HENN NH2=N-2 171=1+1 :1,1+1,...,HH2 1+Sek=148TH SIGHATSQRT(SIGHAR) REFEAT TO 1 11=1x+1 FOR \$16HAZ=\$16HAZ+4(13)##2 C1AG(1) =A(11) P1, IP1+1, . . . 1171=1X+1+1 10 AYES UPPERO(1) = A(11P1) UFFERD(1) =-\$1GN(31GHA, A(11F1)) ABS(SIGNA).GT.ABS(A(IIP1)) 60 70 10 1(1,2)=SIGHAR A(11P1)=0. REPEAT TO 3 SQTGAH=-SIGH(SIGHA+A(IIP1), UPPERD(I)) FOR A(11F1)=SGRT(1.+ABS(A(11F1))/SIGHA) 172=1+2 J=1P2,1P2+1,...,H REFEAT TO 5 JK1=1+(2+H-1-1)/2 YALL(J) =0. FOR A (1 J) = A (1 J) / SQTGAN JX=JX1 JK=JK1+J /=IP1,IP1+1,...,N IIX=JK1 10 60 YES REPEAT TO 4 FOR J.EQ.N VALL (J) = VALL (J) · A (JR) · A (IR P1,1P1+1,... REPEAT TO 7 TRANSFER TO SUBROUTINE LOOFI | | L-M+X! = XL CELGAM: 0. Z, NP3, VALL (J) , A (JX) , A (TX)









STHINV

A IS THE UPPER TRIANGLE OF THE SYMMETRIC MATRIX TO BE INVERTED.

ELEMENTS ARE STORED ROWWISE.

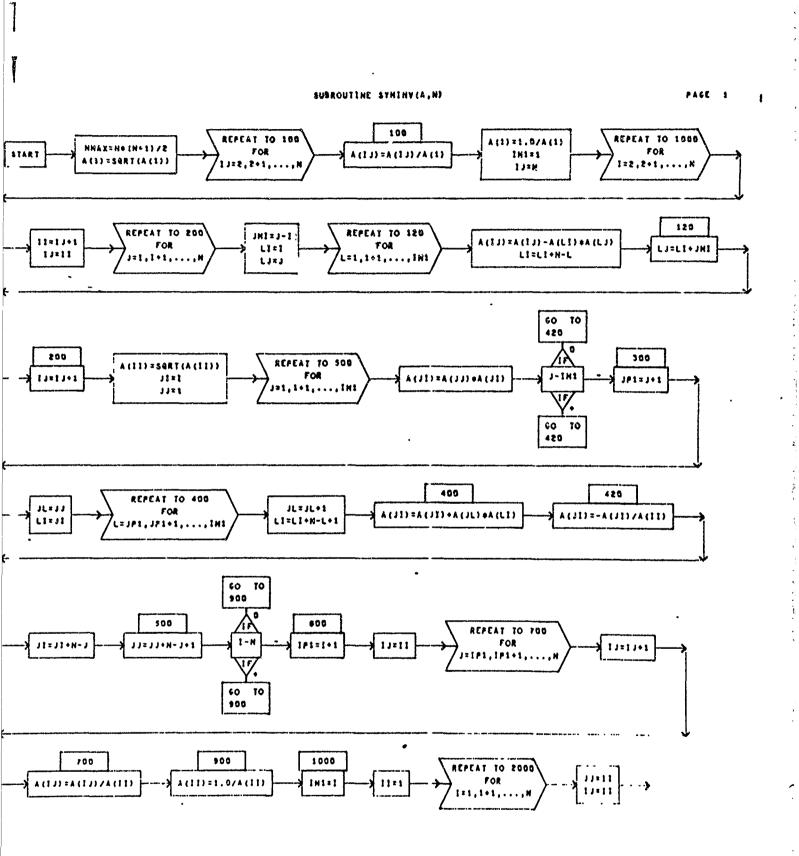
. N = ORDER OF MATRIX

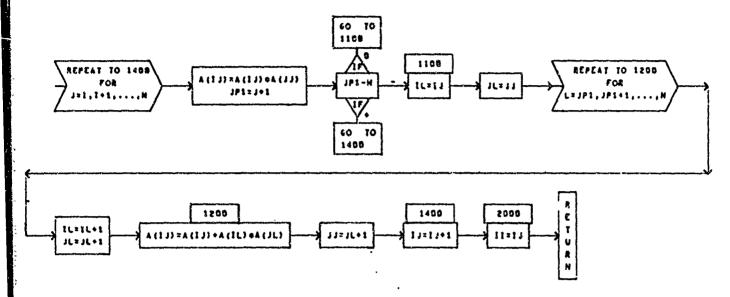
PROGRAM INVERTS IN PLACE.

BINENSIONES VARIABLES

SYMBOL STORAGES SYMBOL STORAGES SYMBOL STORAGES SYMBOL STORAGES

180





ESGHAT 1

THIS SUBROUTINE FINDS THE EIGENFALUES AND EIGENVECTORS FOR SYMMETRIC MASS AND STIFFNESS MATRICES.

THE ARGUNENTS ARE--

H- ORDER OF MATRICES.

A- DUNNY VECTOR WITH DIMENSION IN MAIN PROGRAM OF No (N+1)/2
VALU- STORAGE FOR EIGENVALUES MUST BE DIMENSIONED IN THE MAIN
PROGRAM AS A VECTOR OF LENGTH NEIG.

TEMP, 8, C, D, - DUMNY VECTORS WITH DINENSION OF N IN HAIN FROGRAM.

E- DUMNY ARRAY WITH DIMENSIONS OF (N, 3) IN HAIN FROGRAM.

IDUN- DUMNY INTEGER VECTOR WITH DIMENSION OF N IN HAIN FROGRAM.

MTAPE- TAPE WHERE STIFFNESS MATRIX IS STORED IN COMPACT FORM.

NTAFE- TAPE WHERE MASS MATRIX IS STORED IN COMPACT FORM.

STAPE, STAPE- SCRATCH TAPES.

NEIG- NUMBER OF EIGENVALUES DESIRED.

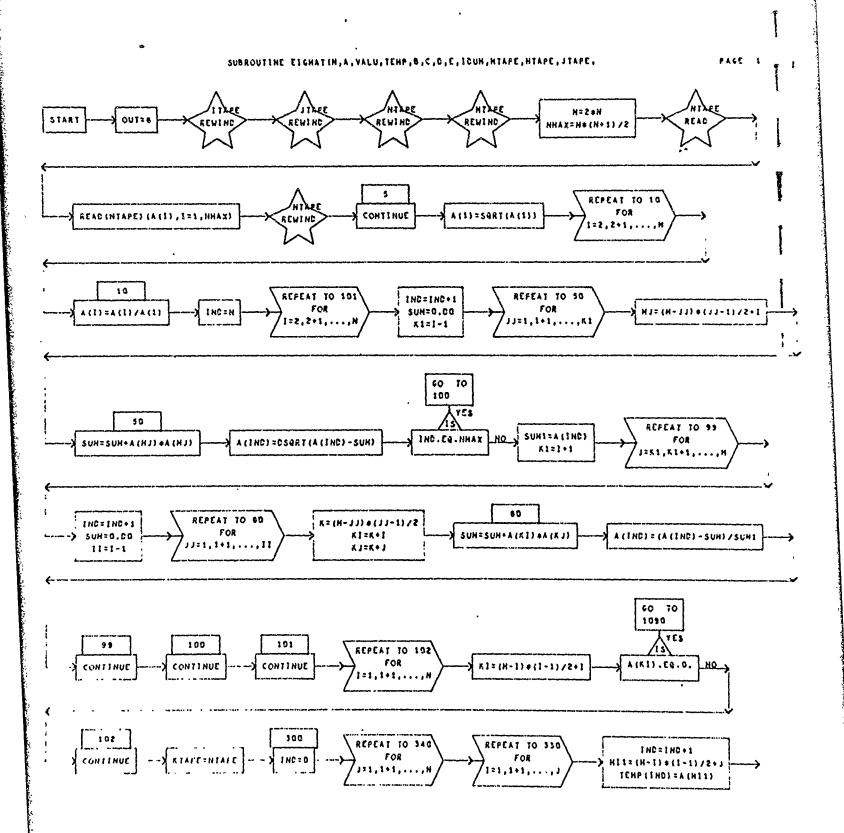
NVEC- NUMBER OF EIGENVECTORS DESIRED. MUST BE EQUAL TO OR LESS

THAN NEIG.

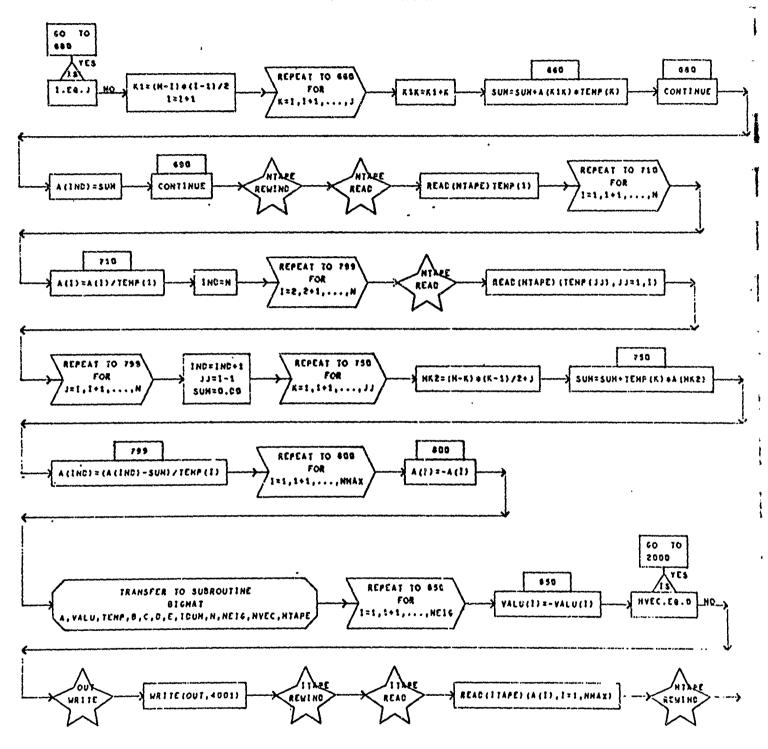
THE HASS AND STIFFNESS MATRICES ARE STORED IN COMPACT FORM AS VECTORS. ONLY THE UPPER TRIANGLE OF THESE MATRICES (BY ROWS) 18 STORED.

CIMENSIONED VARIABLES

JOBKYZ	STORAGES	STHEOL	STORAGES	STHBOL	STORAGES	SYNBOL	STORAGES	STHBOL	STORAGES
A	1	TEHP	1	· VALU	1		t	c	1
c	ı	C	н, 3	IDUM	1				



SUBROUTINE EIGHAT (N.A. VALU, TEMP, D.C.D.E., IDUM, HTAPE, HTAPE, JTAPE, 9 39 A 4 340 330 REPEAT TO 418 WRITE (KTAPE) (TEMP(JJ), JJ=1,1ND) FOR CONTINUE IND=0 1=1,1+1,...H 410 REPEAT TO 499 REPEAT TO 490 FOR FOR IND=(1+(H+3-1))/2-N A (1HC) = 1 . /A (1HC)) j= (N+2) - j 450 REPEAT TO 450 IND= (N+1+1-3) + (11-1)/2 ICK=IHD+K FOR SUH=SUH+A (ICK) #A (HK) SUH=0.00 HK= (H-K) + (K-1)/2+JJ K=K1,K1+1,...,JJ K1=JJ-1+2 499 INC=INC+JJ (XAHA, 2=1, (1) A) (39ATI) 37 IRW A(1HD) =- SUH+A(1C1) CONTINUE 101=1MC-1+1 INCED REPEAT TO 355 REPEAT TO SEE IND=IND+1 FOR #12=(H-1)+(1-1)/2+. CONTINUE TEMP (INC) =A (HIE) 555 WRITE (JTAFE) (TEMP (JJ), JJ=1, INC) CONTINUE REAC (HTAFE) (A (I), I=1, NNAX) REPEAT TO 690 REPEAT TO 699 FOR REFEAT TO 650 HK1=(H-K)+(K-1)/2+1 SUN = 0.00



PL YHP 12/6

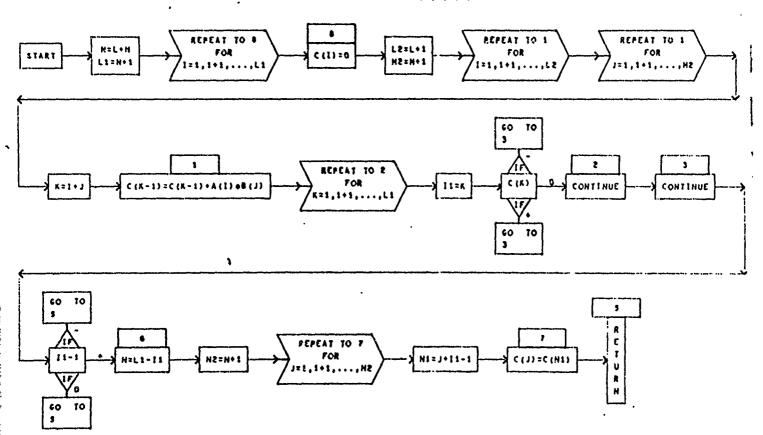
POLYNONIAL HULTIPLY

DINENSIONED VARIABLES

SYMBOL STORAGES SYMBOL STORAGES SYMBOL STORAGES SYMBOL STORAGES SYMBOL STORAGES

SUBROUTINE PLYMP (A, L, B, M, C, N)

PAGE 1



APPENDIX D

Symbol List

Symbol List

Listed below by their FORTRAN names are some of the input quantities to the program and their equivalent names in Section 3.0.

Input Quantity	Symbol in Section 3.0
MY	E
PR	ער
GE	G
DENS	P
X	X
Y	Y
RSMASS	$\mathtt{M}_{\mathbf{i}}$
AR .	A
XI	I
¥Ј	J
PTH	t
DX	$\mathtt{D}_{\mathbf{x}}$
DY	D _y
D1	D_1
DXY	D _{xy}
BETA	ß

Security Classification

DOCUMENT CONT				
1. ORIGINATING ACTIVITY (Corporate author) HUGHES AIRCRAFT COMPANY, MISSILE SYSTEM FALLBROOK AND ROSCOE BLVDS. CANOGA PARK, CALIFORNIA 91304		22. REPORT SCURITY CLASSIFICATION UNCLASSIFIED 25. GROUP		
COLLOCATION FLUTTER ANALYSIS STUDY	• • •			
4. DESCRIPTIVE NOTES (Type of report and inclusive dates) FINAL REPORT (MARCH-1968 THROUGH MARCH : 5. AUTHORIS) (First name, middle initial, lest name) DYNAMICS AND ENVIRONMENT SECTION, DONALI				
6. REPORT DATE APRIL 4, 1969 8. CONTRACT OR GRANT NO. NOO019-68-C-0247 b. PROJECT NO.	Se. ORIGINATOS	·	75. NO. OF REFS	
c. d.	9b. OTHER REPO this report)	RT NO(\$) (Any of	her numbers that may be essigned	
IN ADDITION TO SECURITY PROUIREMENTS UP EACH TRANSMITTAL OF THE CUMENT OF MUST HAVE PRIOU APPROVAL OF THE COMMANDE	E THE ASSIST	IES OF THE	U.S. COMMENT	
TO SUPPLIANT NOTES	2	SYSTEMS CO T OF THE NA	DMMANUS	

THIS STUDY COVERS THE DEVELOPMENT OF A SET OF COMPUTER PROGRAM TO PERFORM
FLUTTER ANALYSIS BY THE COLLOCATION METHOD. WHILE THIS METHOD HAS BEEN KNOWN
FOR SOME TIME, ONLY RECENTLY HAVE ADVANCES IN COMPUTER TECHNOLOGY MADE THE METHOD
TECHNICALLY AND FINANCIALLY FEASIBLE. THE INGREDIENTS OF A COLLOCATION FLUTTER
ANALYSIS ARE 1) A FLEXIBILITY MATRIX, 2) AEMODYNAMIC INFLUENCE COEFFICIENT
MATRIX, AND 3) AN EIGENVALUE SOLUTION. THIS STUDY IS PRESENTED IN FOUR VOLUMES.
VOLUME I CONTAINS A GENERAL PROGRAM DISCUSSION. VOLUME II CONTAINS THE PROGRAM
FILUENC WHICH CALCULATES THE FLEXIBILITY MATRIX. VOLUME III CONTAINS A SET OF
THREE PROGRAMS TO CALCULATE AERODYNAMIC INFLUENCE COEFFICIENTS FOR SUBSONIC,
TRANSONIC, AND SUPERSONIC FLIGHT REGIMES. VOLUME IV CONTAINS THE PROGRAM COPA
WHICH SETS UP AND SOLVES THE FLUTTER EIGENVALUE MATRIX.

DD FORM

END

TOTAL PAGES

TOTAL SHEETS

DOCUMENTS